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EXISTING CONDITIONS REPORT - SERVICING

FOR

STITTSVILLE SOUTH URBAN EXPANSION AREA

CAIVAN (STITTSVILLE SOUTH) INC. & CAIVAN (STITTSVILLE WEST) LTD.

CITY OF OTTAWA

PROJECT NO.: 21-1247

SEPTEMBER 2023 – 1ST SUBMISSION

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1.0 INTRODUCTION

Caivan (Stittsville South) Inc. and Caivan (Stittsville West) Ltd. (Caivan) have retained a Consultant Team to prepare documents to support the Stittsville South Urban Expansion Area (SSUEA) which will be implemented as an Amendment to the City of Ottawa Official Plan (OP) and removal of the Future Neighborhood Overlay on Schedule C17.

Per the City of Ottawa request, a Terms of Reference (TOR) document was prepared and validated by the City (TOR included in Appendix E for reference) to outline the servicing assessment approach for the subject lands. David Schaeffer Engineering Ltd. (DSEL) has been retained to prepare a scoped Master Servicing Study (MSS) to outline water, wastewater, and stormwater management servicing strategies for the SSUEA. In advance of preparing the MSS, an Existing Conditions Report is required to evaluate and assess existing water resources and servicing infrastructure in the vicinity of the SSUEA, and to identify constraints and opportunities that will provide the baseline conditions of an Environmental Management Plan (EMP).

2.0 STUDY AREA

2.1 Location

The properties comprising the Caivan landholdings within the SSUEA are as follows and illustrated in Figure 1:

- ~18.8 ha 6115 Flewellyn Road;
- ~16.1 ha 6070 Fernbank Road;
- ~17.4 ha 5993 Flewellyn Road
- ~12.4 ha (6030 Fernbank Road) parcel and
- ~8.8 ha of holdout land parcels (including Hydro corridor owned lands west of Faulkner Drain) within the SSUEA study area.

The noted land parcels are now designated as Urban Expansion Area in the City of Ottawa Official Plan as of November 2022. As illustrated in the following figure, the overall development area is bound by Flewellyn Road to south, Shea Road to the east, an existing urban subdivision development to the north (Stittsville South – Area 6 (“Edenwyde”)) and an estate lot subdivision (Woodside Acres) to the west.

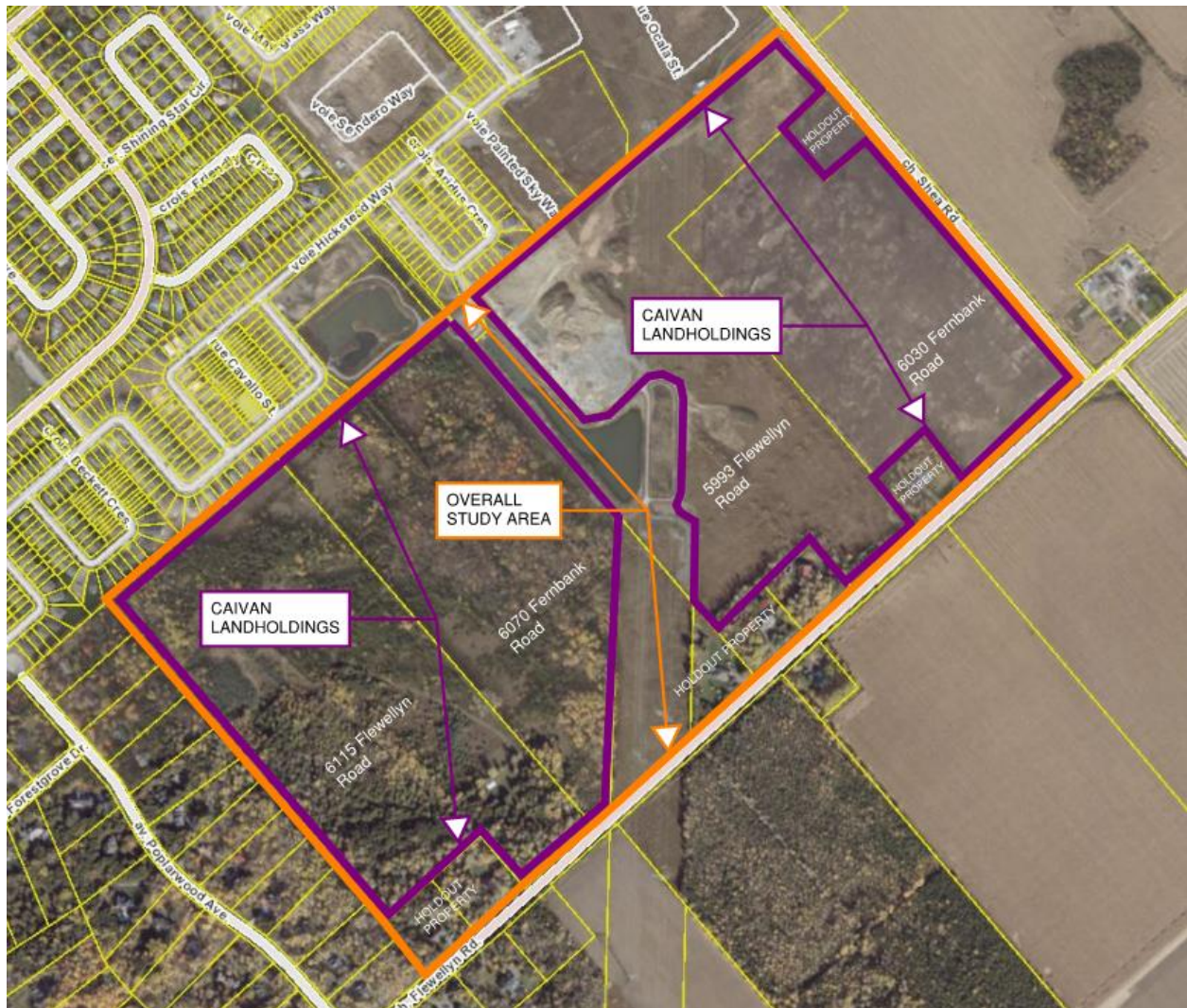


Figure 1: Stittsville South Urban Expansion Area – Location Plan

2.2 Site Characteristics

The subject site is currently undeveloped and is a mix of active/former farmland in the eastern areas and forested areas in the western portion. The overall area is bisected diagonally (north/south) by an existing a Hydro One 500kV utility corridor and an existing stormwater management facility is located centrally within the property and manages flows from a portion of the Edenwyld Subdivision.

In addition, a stormwater conveyance ditch originating from the development areas to the north, runs southward parallel to the east boundary of the Faulkner Property. The ditch officially transitions to being the Faulkner Municipal Drain (FMD) approximately 215 m north of Flewellyn Road and then conveys flows eastward along Flewellyn Road.

For the land parcel west of the Hydro corridor the terrain generally trends lower from northwest to southeast with elevations ranging from 109 m to 103 m. For land area east of

the Hydro corridor the same southeast trend existing with elevations ranging from 104 m to 102 m at Flewellyn Road. The various Existing Conditions figures provided in the Appendices demonstrate the SSUEA site contours.

3.0 BACKGROUND DOCUMENTS

There are a variation of documents and reports that have been prepared in relation to lands surrounding the site. The documents include subwatershed studies of the Jock River, covering the areas south of the site, and servicing documents for the urban area north of the site.

- City of Ottawa Sewer Design Guidelines (City of Ottawa, October 2012) & Technical Bulletins (ISDTB-2014-01, PIEDTB-2016-01, ISTB-2018-01, ISTB-2018-04 & ISTB-2019-02).
- City of Ottawa Water Distribution Guidelines (City of Ottawa, July 2010) & Technical Bulletins (ISD-2010-2, ISDTB-2014-2, ISTB-2018-02, & ISDTB-2021-03).
- Infrastructure Master Plan (City of Ottawa, 2013).
- West Urban Community – Wastewater Collection System Master Servicing Plan (RV Anderson Associates Ltd, July 2012).
- Stittsville Master Drainage Plan (A.J. Robinson, 1994).
- Low Impact Development Technical Guidance Report, Implementation in Areas with Potential Hydrogeological Constraints (Dillon Consulting and Aquafor Beech, February 2021).
- Stormwater Planning and Design Manual (Ministry of the Environment, March 2003).
- Low Impact Development Stormwater Management Guidance Manual (Ministry of the Environment, Conservation and Parks, Draft for Consultation – January 2022).
- Jock River Reach 2 & Mud Creek Subwatershed Study Marshall Macklin Monaghan / WESA, May 2009
- Amendment to the Engineer’s Report for the Faulkner Municipal Drain (Robinson, December 2020) & Addendum No. 1 (Robinson, March 2021).
- Engineer’s Report for the Flowing Creek Municipal Drain (A.J. Graham Engineering, December 1973).
- Flowing Creek Flood Risk Mapping from Flewellyn Road to Jock River (Rideau Valley Conservation Authority, May 8, 2017).
- Stittsville South – Area 6, City of Ottawa, Master Servicing Report & Stormwater Management Design Plan (Novatech/DSEL, December 2013).
- Stittsville South Subdivision, City of Ottawa – Detailed Servicing & Stormwater Management Report (Novatech July 2016).
- Sanitary Pump Station Pre-Design Report, Stittsville South (Novatech, July 2015)
- Stittsville South Subdivision, City of Ottawa – Shea Road Sanitary Pump Station Design Brief (Novatech, May 6, 2016).

- Design Brief, Davidson Lands – OPA 76 Area 6a, Phase 1 (5993 Flewellyn Road) (IBI Group, February 2018).
- Design Brief, Davidson Lands – OPA 76 Area 6a, Phase 2 (5993 Flewellyn Road) (IBI Group, July 2020).

- Design Brief for the Stormwater Management Pond for the Davidson Lands (JFSA/DSEL, November 2017).
- Fernbank Community Design Plan, Master Servicing Study (Novatech, June 2009).
- Geotechnical Investigation, Proposed Residential Development 5993, 6070 & 6115 Flewellyn Road – Ottawa (Paterson (PG5570-2) January 2022).

4.0 EXISTING INFRASTRUCTURE AND SERVICING

4.1 Wastewater Servicing

A recently (2017) constructed sanitary pump station (Stittsville South Area 6 Sanitary Pumping Station – also referred to as the Shea Road Pump Station (SRPS)) associated with the recent new development to the north is located along the north boundary centrally to the land parcels being reviewed (immediately north of an existing stormwater management pond (Davidson Pond)). As per the Environmental Compliance Approval (ECA #3415-ADWLJG issued September 21, 2016. See **Appendix B**) for the pump station, the initial firm capacity of the station was 42 L/s with recently completed pump expansions to an ultimate firm design capacity of 84 L/s (December 2022).

A 450mm diameter sanitary sewer connects to the existing Shea Road sanitary pump station. The existing 450mm diameter inlet sanitary sewer has an inlet pipe elevation of 98.72 m at the last manhole upstream of the station and wet well invert elevation of 96.50 m. The inlet sewer has a residual capacity of approximately 80% which would allow for an additional ~390 L/s of sanitary flows.

Previously, the SRPS directed forcemain flows northward to the existing Liard Street pumping station. As of December 2022 the SRPS now directs flows directly to the recently completed extension of the Fernbank Lands trunk sanitary sewer. Additionally, there is an existing low lift pumping station located on nearby Friendly Crescent which provides service to 70 dwellings and discharges to the Hartsmere Drive sanitary sewer through a 100mm diameter forcemain. Ultimately this low lift station will be decommissioned and the sanitary flows redirected to the SRPS. Timing for this decommissioning is still pending.

As per the Master Servicing Report for the *Stittsville South – Area 6* development, there is excess capacity available in the Fernbank Lands trunk sewer (see report excerpt in **Appendix B**). The *Area 6* study summarized that the Fernbank Trunk was designed for a peak flow of 528 L/s (*Fernbank CDP Lands – New Trunk Sewer* sanitary design sheet provided in **Appendix B** for reference) and had a capacity of 670 L/s (excess capacity of 142 L/s). The *Area 6* report further summarized that the *Area 6* and Liard Street P.S. (monitored) flows to the Fernbank Trunk totaled approximately 85 L/s and 39 L/s respectively and would utilize a portion of this capacity. However, the original design criteria of the Fernbank Trunk system (and *Area 6*)

was based on older City of Ottawa design criteria. When considering the new criteria adopted by the City after those designs the excess capacity available is increased.

**Table 1
 Sanitary Flow Review**

Network Reviewed	Area (ha)	Pop.	PF (7)	Q _{units} (L/s)	Q _{Com/Inst} (L/s)	Q _{I/I} (L/s)	Q _{TOT} (L/s)	Diff. (L/s)
Old City Parameters for Sanitary (1)								
Fernbank CDP Lands (2)	551.8	30,169	2.47	302.5	71.0	154.5	528.0	--
Stittsville Area 6(3)	70.74	4,502	3.29	59.94	2.37	19.81	82.1	--
Liard St P.S. (monitored) (4)	--	--	--	--	--	--	39.0	--
New City Parameters for Sanitary (5)								
Fernbank CDP Lands	551.8	30,169	2.18	213.1	39.76	182.09	435.0	-93.0
Stittsville Area 6	70.74	4,502	2.83	41.2	1.33	23.34	66.0	-16.1
Liard St P.S. (monitored) (6)	--	--	--	--	--	--	39.0	0
(1) Old City Parameters: 350 L/day; 0.28 L/s/ha infiltration; Comm./Inst. Flow = 50,000 l/ha/day (2) Sanitary design sheet excerpt provided in Appendix B. From "Fernbank Community Design Plan – Master Servicing Study (June 2009)" (3) Sanitary design sheet excerpt from updated IB design for Edenwyld development. From City submission 2020-04-09" (4) Liard Street pump station – monitored flow summary from the "West Urban Community – Wastewater Collection System Master Servicing Plan" by RV Anderson Associates Ltd., dated July 2012 and as summarized in the Area 6 MSS. (5) New City Parameters: 280 L/day; 0.33 L/s/ha infiltration; Comm./Inst. Flow = 28,000 l/ha/day; updated Peak Factor correction factor (6) Same value as prior as it was monitored information. (7) Peaking Factor								

From the table above the flow summarized in the Fernbank Lands trunk is reduced from 528.0 L/s to ~435.0 L/s (-93.0 L/s) based on review with new parameters. The Area 6 land development flows are reduced from 82.1 L/s to ~66.0 L/s (-16.1 L/s).

The Area 6 MSS summarized excess capacity at peak flow in the Fernbank Lands trunk at 142 L/s. With the new parameters this excess capacity increases to 235 L/s based on the above table with 105 L/s of that taken up by the Area 6 and the Liard St. P.S. flows (130 L/s capacity remaining).

Construction of the Fernbank Lands trunk extension up to the Area 6 development was completed/commissioned in December 2022.

The SRPS details are as follows:

- pump station control building complete with mechanical and electrical systems, process piping, valves, control panels, SCADA system, odour control system, swab launchers and appurtenances;
- one (1) 2400 mm diameter FRP wet well, complete with valves, couplings and appurtenances; with three (3) pumps with each pump capable of delivering 42

- liters/second at a TDH of 29 meters for an ultimate firm capacity of 84 liters/second;
- wastewater flows are pumped via dual 200 mm diameter HDPE DR13.5 sanitary forcemains to a to a new discharge chamber on Fernbank Road outletting flows to the newly constructed Fernbank Sanitary Trunk Sewer (completed/commissioned in December 2022);
 - one (1) 2400 mm x 1800 mm concrete by-pass chamber, complete with valves, couplings and appurtenances;
 - one (1) 1800 mm diameter concrete by-pass manhole, complete with valves, couplings and appurtenances;
 - one (1) 1800 mm diameter concrete emergency overflow manhole, complete with one (1) primary measuring device consisting of broad crest weir complete with ultrasonic level recorder (referred to as SAN MH 97);
 - one (1) concrete encased underground dedicated commercial hydro service;
 - one (1) 170 KW self-enclosed diesel generator on a reinforced concrete pad adjustment to the pump station control building, complete with diesel fuel tank, valves and controls;
 - emergency sanitary sewer overflow consisting of a 600 mm diameter sewer to the adjacent Davidson Stormwater Management facility located south of the SRPS (outlet elevation 103.40 m).

4.2 Water Supply Servicing

4.2.1 Existing Water Supply Services

The SSUEL study area will be part of the City's Zone 3W of the City of Ottawa water distribution network (see Drawing 3 for reference). The pressure zone receives supply from the Campeau Drive and Glen Cairn Pump stations. The Stittsville Elevated Tank provides balancing storage during peak usage and fire flow conditions.

Existing watermains to the north of the subject lands represent the only option for water servicing. These include:

- The major water supply line in the vicinity of the development is a 400mm diameter watermain along Fernbank Road;
- An existing 250mm diameter watermain located within the Parade Drive right-of-way (ROW), immediately north of the Maguire and Faulkner land parcels. A future southbound ROW block is located between civic addresses 714 and 720 Parade Drive;
- An existing 250mm diameter watermain is located within the Aridus Crescent ROW which is north of the Davidson Lands parcel. An existing 50mm water service within a servicing block from Aridus Crescent to the SRPS pump station is also installed facilitating water supply to that facility;
- An existing 200mm diameter watermain located within the Painted Sky Way ROW at the northwest portion of the Davidson land parcel; and
- An existing 200mm diameter watermain location within the Ocala Street ROW north of the northeastern portion of the Davidson land parcel.

4.2.2 Existing Watermains and Operating Pressures

In relation to the Stittsville Area 6 development areas to the north, the water supply was reviewed for two separate analyses:

1. Stantec Consulting Ltd. prepared a hydraulic analysis of the proposed western portion of the Area 6 lands in their report titled "Stittsville Area 6 – Phase 1 & 2 – Potable Water Hydraulic Assessment (September 2, 2015). This model was based on the City up to date model that was updated for the 2013 Water Master Plans with current (in 2015) conditions and future conditions (projected 2031 conditions from the 2013 Water Master model) analyzed.
2. IBI Group prepared a hydraulic analysis as part of their "Design Brief – Davidson Lands – OPA 76 Area 6a, Phase 1" (February 2018) servicing reporting for the eastern portion of the Stittsville Area 6 lands. This analysis was based on boundary conditions provided by the City of Ottawa (see report excerpts in **Appendix C**).

The Stantec analysis above notes that head losses under peak demands could reduce minimum pressure to below guideline requirements at higher elevations (i.e. ground elevations greater than 124m). However, future planned connections within the Fernbank Lands development area will mitigate the issue.

During average day demands ground elevations less than 106m may experience system pressures greater than the upper 80psi limit specified in City guidelines. As noted in Section

2.2, existing site elevations range from 109 m to 103 m (for areas west of the Hydro One corridor) and from 104 m to 102 m in the eastern areas. Should higher pressures be encountered in the southern areas of the development pressure reducing valves would be required.

Water servicing needs in the SSUEA will be evaluated as part of the future MSS review of the development area in consultation with City staff via the generation of hydraulic boundary conditions.

4.3 Stormwater Servicing

4.3.1 Parade Drive Stormwater Management Facility

The residential development area to the north of the Maguire/Faulkner properties is serviced via an existing 1.9 ha stormwater pond block adjacent to Parade Drive. This stormwater facility has the following characteristics:

Drainage Area = ~33.7ha
Permanent Pool Elevation = 103.50 m
Extended Detention Elevation = 103.70 m
100-Year Elevation = 105.33 m

The facility outlets to an existing ditch located east of the storm outlet. The ditch is approximately 405 m upstream of the commencement of the Faulkner Municipal Drain.

4.3.2 Davidson Stormwater Management Facility

The existing development to the north of the Davidson/Eder properties is serviced by the central “Davidson” stormwater management pond. The existing Davidson stormwater pond occupies approximately 3.2 ha of land and is partially located under the existing Hydro One tower line. The ponds are sized for their respective areas with no specific additional areas considered. This stormwater facility has the following characteristics:

Drainage Area = ~40.6 ha
Permanent Pool Elevation = 101.50 m
Extended Detention Elevation = 102.10 m
100-Year Elevation = 103.17 m

The facility outlets from the south end of its configuration to a ditched outlet that conveys the flows southwest to the Faulkner Municipal Drain.

4.3.3 Faulkner Municipal Drain

The Faulkner Municipal Drain (FMD) generally bisects the whole of the development area in half. The FMD conveys flows from north to south to the north side of Flewellyn Road (i.e. roadside ditch) then heads eastward and then southwards along the west side of Shea Road. The FMD drain begins at approximately 215 m north of Flewellyn Road (within the development lands) and ultimately discharges to Flowing Creek Municipal Drain 5.45 km away (approximately 330 m south of the intersection of Shea Road and Brownlee Road). Figure 6.1 (*Maintenance Sections and Section Drainage Areas*) from the Engineer’s Report is provided in **Appendix D** for reference.

The Engineer’s Report for the FMD was recently amended in December 2020 by Robinson Consultants Inc. to accommodate the changes in land use from rural, or agricultural, to urban development. Additionally, some modifications of the main drain were also documented in order to relocate a portion, lower the profile in some locations, and modify the cross-section where required in order to increase capacity and reduce erosion potential. No specific erosion thresholds are noted for the FMD in the Engineer’s Report.

Subsequent to the amended Report, there was a minor addendum in March 2021 to account for an adjustment in the prescribed value for lands utilized for construction of the drain and the resultant modified value of allowances.

The FMD model will be utilized during the design of future stormwater management facilities to confirm that there will be no negative impacts to water levels or capacity of the drain in the post-development condition. JFSA has reviewed the existing conditions as a component of their “*Pre-Development Hydraulic and Hydrologic Study*” (provided in **Appendix D**) and noted some private access culverts along Flewellyn Road are close to or at capacity as summarized in the Engineer’s Report Table 4.2. These culverts can be revisited in association with future consultation with the Drainage Engineer in association with the advancement of the development area.

4.3.4 Flowing Creek Municipal Drain

As noted in the prior section, the FMD outlets to the Flowing Creek Municipal Drain (FCMD) south of Brownlee Road. The Engineer’s Report for FCMD was prepared by A.J. Graham Engineering Consultants Limited in December 1973 and was constructed in 1974 by the former Township of Goulbourn. There are no known issues with the FCMD.

The Rideau Valley Conservation Authority completed Flood Risk Mapping for Flowing Creek in May 2017 (covering from Flewellyn Road to the confluence with the Jock River). The RVCA report makes reference to some possible shallow field flood areas southwest of Akins/Shea Road, however, it goes on to detail the ‘considerable uncertainty’ as to how this may occur and whether there would be any material impact to the adjacent FMD (see RVCA report excerpts in **Appendix D** for reference). However, it is presumed that the Drainage Engineer for the FMD has considered this potentiality based on their recent FMD improvements and knowledge of the FCMD Flood Risk Mapping results.

4.3.5 Site Drainage

For the Caivan landholdings west of the Hydro one corridor the site topography generally drains eastward and southward with drainage ultimately being conveyed to the FMD which bisects the development area.

Similarly, the development area east of the Hydro One corridor also drains eastward and southward to the portion of the FMD along the northern Flewellyn Road right-of-way.

JFSA has reviewed the development area’s existing conditions as a component of their “*Pre-Development Hydraulic and Hydrologic Study*” provided in **Appendix D** for reference.

There are no minor storm sewer systems that the development area is tributary to.

5.0 OPPORTUNITIES AND CONSTRAINTS

5.1 Drainage Network

The review of site topography has generally shown that surface water is conveyed to adjacent perimeter roadside ditches and the Faulkner Municipal Drain.

As a component of the review of storm servicing for the future MSS, any adjustment to drainage boundaries or outlets will require consultation with appropriate agencies. Generally speaking, the development only has one viable stormwater outlet which is the FMD and stormwater management facilities will be located at the southern boundary of study area due to site topography. Therefore any adjustments will need to be coordinated with the Drainage Engineer and any processes completed in accordance with the Drainage Act. This includes consideration of the Flowing Creek Municipal Drain (FCMD) (which the FMD connects to) and ultimately the Jock River.

Lands which comprise the SSUEA are not restricted by floodplain areas from any major watercourses. Preliminary review of the FMD HEC-RAS modelling associated with the recently updated Engineer's Report (see Section 4.3.3) has indicated that there may be private access culverts on the FMD along Flewellyn Road that could constrain flows during the 100-year event and these will be further assessed in future design stages in consultation with the Drainage Engineer.

5.2 Water Quantity Control

Water quantity controls for the development area will be impacted by various site constraints (i.e. infiltration potential, development density, etc) as well as downstream capacities. At minimum, post-development peak flows within the FMD are not to exceed pre-development levels for all storms up to the 100-year event. Generally this will require review of the FMD and FCMD based on the on-site controls implemented and also manage runoff volumes so as not to create downstream impacts. Prior consultation with the RVCA for other development areas tributary to the Jock River has indicated that there are no quantity control required within the Jock River Reach 2 subwatershed. Updated subwatershed reporting is currently a work in progress.

5.3 Water Quality Control

Water quality control for the development area will have to be in accordance with the Jock River Reach 2 & Mud Creek Subwatershed Study. Similar to the adjacent development areas recently advanced, this would mean that the requirement is for an enhanced protection level (80% TSS removal) of water quality treatment.

5.4 Infiltration

The Hydrogeological review completed by Paterson Group characterized the hydrogeological condition of the SSUEA with respect to bedrock and surficial geology, aquifers, aquitards, horizontal and vertical flow patterns, existing groundwater use, and aquifer vulnerability. The report generally summarizes that the overburden and bedrock within the SSUEA have hydraulic conductivity values ranging from 4.2×10^{-6} m/sec to 2.2×10^{-5} m/sec (moderate hydraulic conductivity) and 4.3×10^{-7} m/sec to 1.6×10^{-4} , respectively (refer to the *Table 2* summary from the Paterson report in **Appendix D**). Field saturated conductivity values from Paterson's *Table 3* are also provided. Highest surficial field saturated values were observed

within the southwestern portion of the subject site indicating that this area will have more permeable characteristics than the northeastern areas and as such are more conducive for providing LID measures for water balance and could be considered for optimizing the rate of infiltration via typical lot level and conveyance Best Management Practices (BMPs).

JFSA has reviewed the development area's existing conditions as a component of their "*Pre-Development Hydraulic and Hydrologic Study*" provided in **Appendix D** for reference. JFSA's water budget modelling considered the shallow infiltration results, as reported by Paterson, in their analysis. The JFSA detailed PCSWMM model was run for 39 year, from 1967 to 2007, using hourly rainfall data from Environment Canada's Ottawa International Airport monitoring station. Table 1 from the JFSA report are provided in **Appendix D** for reference. The table outlines the water budget breakdown of the development area of the SSUEA. Based on the simulations, JFSA assessed that the eastern portion of the development area (east of the Faulkner Drain) will have 17% of the annual rainfall resulting in runoff with 63% evaporating and 20% infiltrating.

Mapping from the Mississippi-Rideau Source Protection Plan indicates that some portions of the development area may fall under the fringes of the Significant Groundwater Recharge Area (SGRA) mapping. The site review by Paterson has indicated that the high Rock Quality Designation (RQD) of the bedrock within the site area supports an interpretation that the significance of the recharge to the bedrock aquifer is minimal.

5.5 Existing Servicing Infrastructure

The following opportunities and constraints have been identified for the SSUEA and will be reviewed in further detail in a future MSS.

5.5.1 Wastewater Servicing

Wastewater servicing for the SSUEA is governed by the capacity of the SRPS (and its forcemains) and ultimately by the available residual capacity in the existing recently completed Fernbank Lands Sanitary Trunk sewer. As per the Master Servicing Report for the Stittsville South – Area 6 development, there is excess capacity available in the Fernbank Lands trunk sewer. The Area 6 study summarized that the Fernbank Trunk was designed for a peak flow of 528 L/s and had a capacity of 670 L/s (excess capacity of 142 L/s). The prior sanitary system flows were based on older City of Ottawa parameters. When evaluations flows based on updated parameters in Technical Bulletin ISTB-2018-01 the excess capacity is theoretically increased to 235 L/s.

The existing SRPS also has an emergency overflow outlet (internal weir elevation of 103.40m – see Novatech Drawing No. 113004-PS-SVC in **Appendix B**) to the adjacent "Davidson" stormwater management pond to the south. This overflow will have to be assessed at detailed design to determine if a new overflow is required based on projected underside of footing elevations during the future MSS preparation.

As noted in Section 4.1 the existing sanitary sewer inlet at the SRPS is at an invert elevation of 98.72m. Existing ground elevations in the southeast portion of the SSUEA are as low as ~101.60m which imposes some constraint in terms of fill import required to facilitate a gravity system that would convey sanitary flows all the way to the SRPS approximately 1km away.

5.5.2 Water Servicing

Based on prior analyses undertaken for development areas within Area 6 to the north of the subject site, lower water pressures are anticipated during peak hours in areas with ground elevations of 124m or higher and high pressures during average day demands for areas with ground elevations lower than 106m. A future detailed analysis to be prepared in conjunction with the MSS will determine where mitigation may be required within the watermain network.

5.5.3 Stormwater Servicing

There are currently no existing or planned stormwater management facilities associated with the SSUEA lands. The overall site currently drains to the Faulkner Municipal Drain via sheet drainage and various periphery roadside drainage ditches. The FMD poses a constraint for the development area given that it bisects the central portion of the development area while topography for the lands areas on either side have the same northwest to southeast drainage pattern. This drainage pattern does provide the opportunity to have a wet pond facility adjacent to the FMD as an outlet(s) at the southern boundary of the development areas but detailed review during MSS preparation will fully assess whether one or two facilities would ultimately be required.

6.0 SUMMARY AND CONCLUSIONS

6.1 Wastewater Servicing

The sanitary flows from the SSUEA will require conveyance by sanitary pumping to convey flows to the Fernbank Lands Trunk sewer. Gravity flows to the existing SSUEA are possible but are likely constrained by the extent of fill importation required to facilitate sufficient sewer cover on a gravity system in the lower (southern) areas of the SSUEA based on site topography. This includes having appropriate freeboard over the sanitary overflow for the SRPS.

The MSS should evaluate proposed servicing alternatives should fill importation be deemed excessive (i.e. new pumping facility, relocation of the SRPS, etc). In addition, the MSS should review the SRPS sanitary overflow condition and assess whether a new overflow elevation to another location (i.e. a new SWM facility) is warranted to further mitigate site grading conditions.

6.2 Water Servicing

At the Master Servicing Study stage the water supply for the SSUEA will be assessed via a hydraulic assessment of the proposed distribution network in order to confirm sufficient water supply is available, and within the required pressure ranges, under future demands during average day, peak hour and fire flow conditions. Watermain boundary conditions will be requested from the City of Ottawa and the analysis will be completed in accordance with the most current design guidelines and technical bulletins.

6.3 Stormwater Servicing

There are currently no planned stormwater management facilities associated within the SSUEA. The local drainage for the subject area is ultimately conveyed by the FMD which bisects the SSSUEA lands.

The site topography has natural gradients from the northwest to the southeast lending to the practical implementation of stormwater management facility/facilities in the southern areas of the site, with an outlet to the FMD. It is anticipated that any proposed facility/facilities could provide both quantity and quality control to meet required targets. Quantity control would be required to maintain the integrity of the FMD and mitigate any increases in water levels within that system. The MSS will detail the target requirements of the facilities.

A review of options for adjustments to the FMD, where it bisects the site, can be reviewed as part of the MSS but there appears to be limited opportunities due to the location of the FMD in relation to holdout properties not under control by the proponent.

Analyses completed by Paterson Group and JFSA have shown areas of moderate hydraulic conductivity. The MSS will quantify the post-development water balance to summarize that a water balance condition is met.

Prepared by,
David Schaeffer Engineering Ltd.

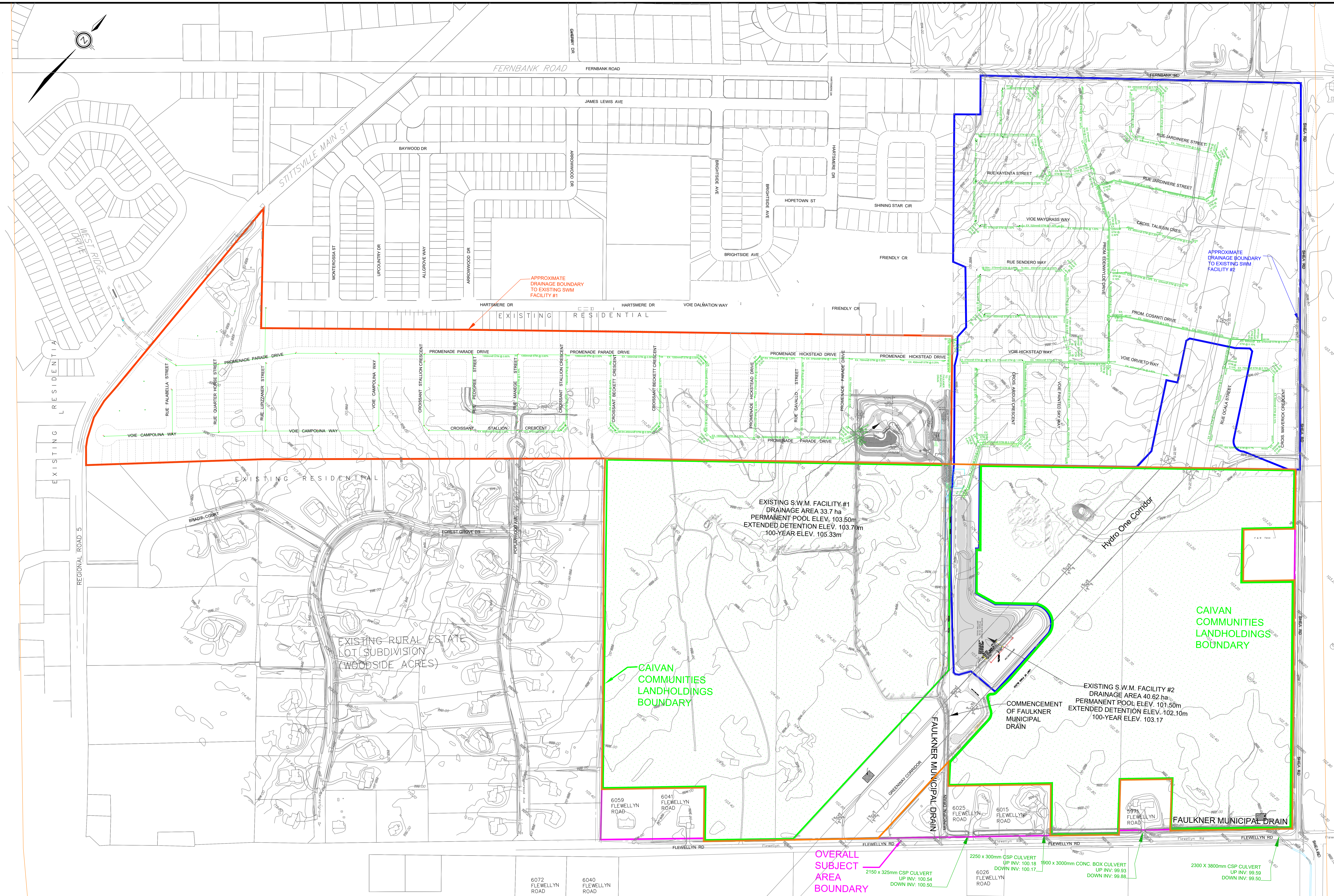
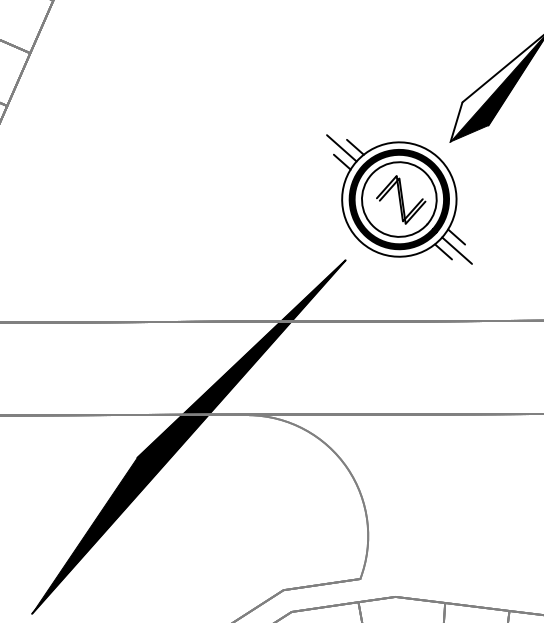


Per: Kevin L. Murphy, P.Eng.

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APPENDIX A

DRAWINGS



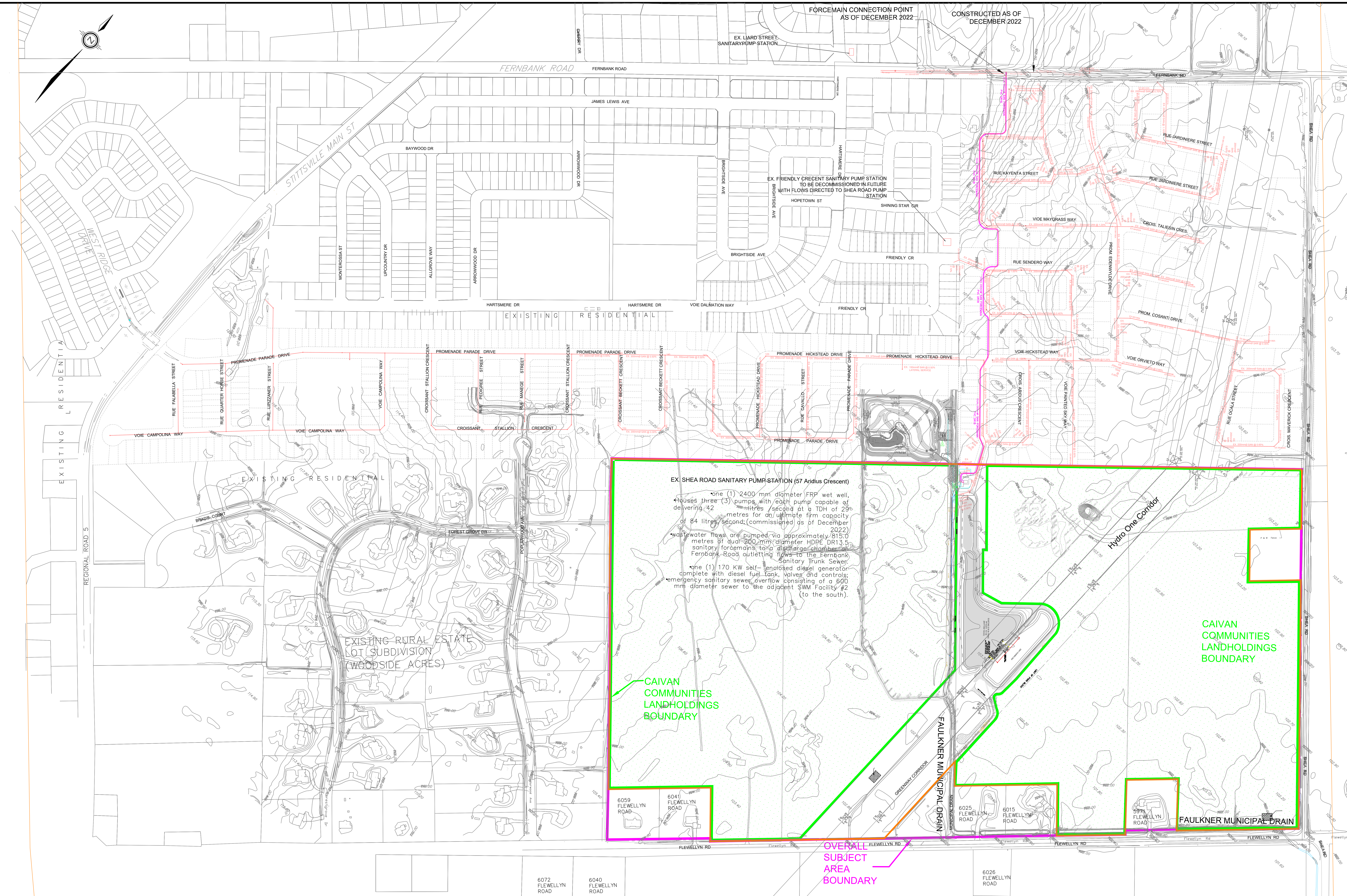
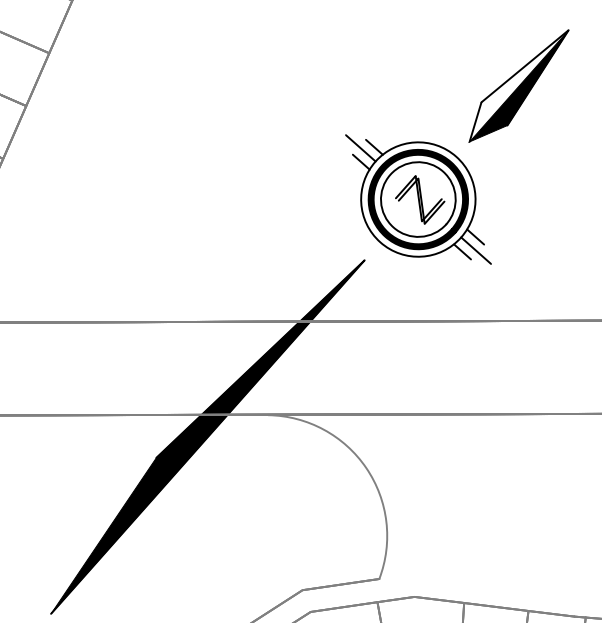
120 Iber Road, Unit 103
 Stittsville, Ontario, K2S 1E9
 Tel. (613) 836-0856
 Fax. (613) 836-7183
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STITTSVILLE SOUTH URBAN EXPANSION AREA
STORM SERVICING EXISTING CONDITIONS
 CITY OF OTTAWA

LEGEND

- EXISTING STORM MANHOLE
- EXISTING STORM SEWER
- OVERALL SUBJECT AREA BOUNDARY
- CAIVAN LANDHOLDINGS

PROJECT No.:	21-1247
DATE:	Sep 2023
SCALE:	1:2000
DRAWING:	1



EX. SHEA ROAD SANITARY PUMP STATION (67 Ardius Crescent)

- one (1) 2400 mm diameter FRP wet well, houses three (3) pumps, with each pump capable of delivering 42 litres per second at a TDH of 29 metres for an ultimate firm capacity of 84 litres per second, (commissioned as of December 2022)
- wastewater flows are pumped via approximately 815.0 metres of dual 200 mm diameter HDPE DR13.5 sanitary forcemains to a discharge chamber on Fernbank Road outletting flows to the Fernbank Sanitary Trunk Sewer.
- one (1) 170 kW self-generated diesel generator complete with diesel fuel tank, valves and controls; emergency sanitary sewer overflow consisting of a 600 mm diameter sewer to the adjacent SWM Facility #2 (to the south).

LEGEND

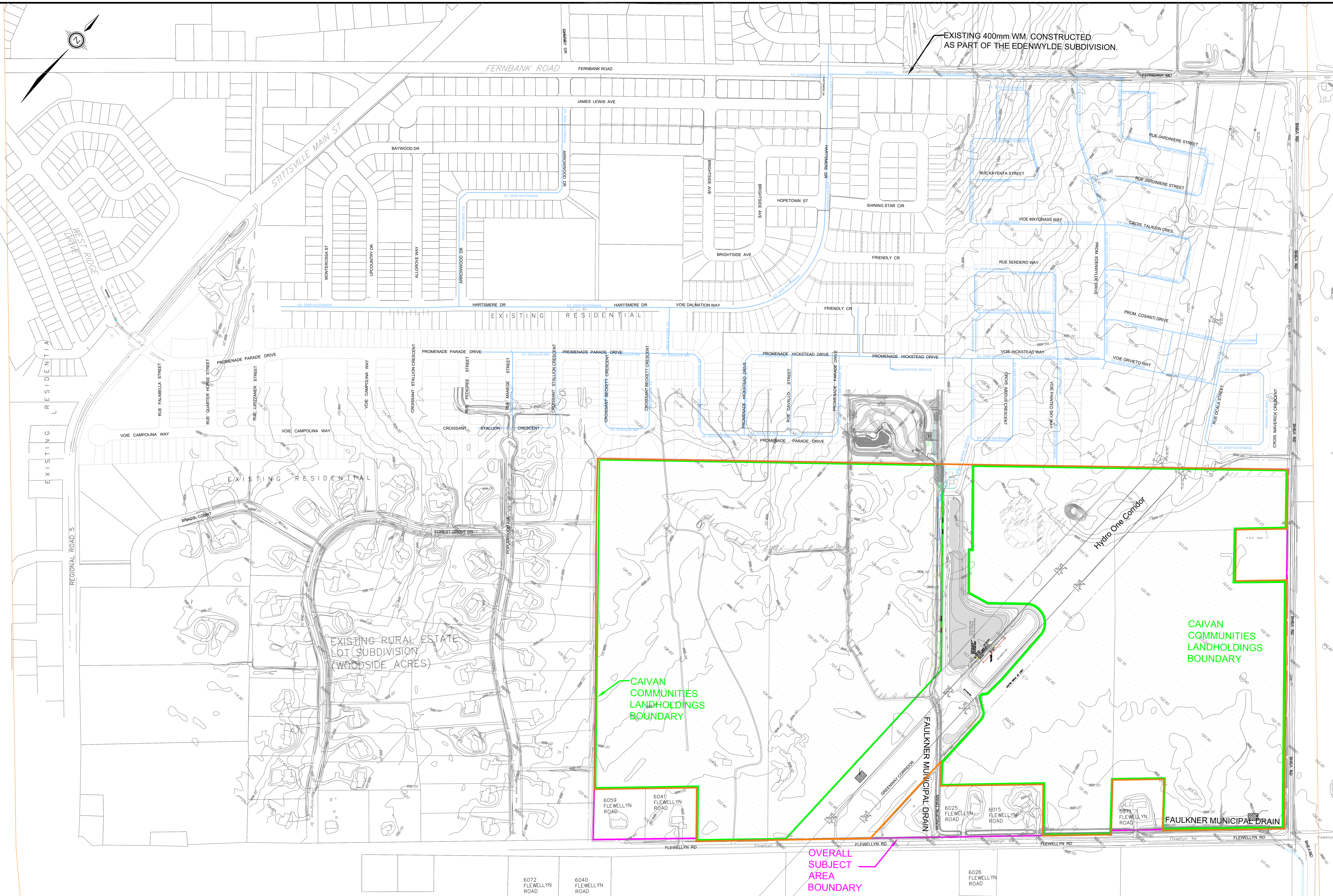
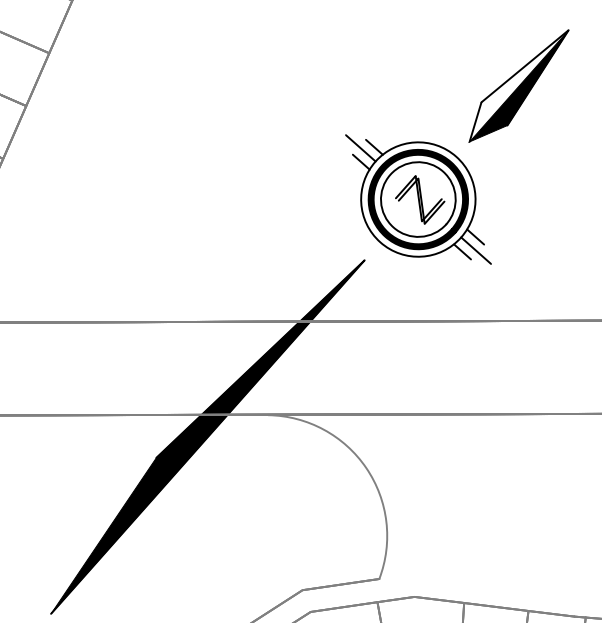
- EX. SANITARY MANHOLE
- EX. SANITARY SEWER
- EX. SANITARY FORCEMAIN
- OVERALL SUBJECT AREA BOUNDARY
- CAIVAN LANDHOLDINGS

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STITTSVILLE SOUTH URBAN EXPANSION AREA
SANITARY EX. CONDITIONS
 CITY OF OTTAWA

PROJECT No.:	21-1247
DATE:	Jan 2023
SCALE:	1:2000
DRAWING:	2





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STITTSVILLE SOUTH URBAN EXPANSION AREA
WATERMAIN EXISTING CONDITIONS FIGURE
 CITY OF OTTAWA

LEGEND

- EXISTING WATERMAIN
- OVERALL SUBJECT AREA BOUNDARY
- CAIVAN LANDHOLDINGS

PROJECT No.:	21-1247
DATE:	Sep 2023
SCALE:	1:2000
DRAWING:	3

APPENDIX B

WASTEWATER

ENVIRONMENTAL COMPLIANCE APPROVAL

NUMBER 3415-ADQLJG

Issue Date: September 21, 2016

Stittsville South Inc. and 1384341 Ontario Ltd.
1737 Woodward Drive, 2nd Floor
Ottawa, Ontario
K2C 0P9

Site Location: Stittsville South Area 6 Sanitary Pumping Station
5970 Fernbank Road and part of 5993 Flewellyn Road
City of Ottawa, Ontario

You have applied under section 20.2 of Part II.1 of the Environmental Protection Act, R.S.O. 1990, c. E. 19 (Environmental Protection Act) for approval of:

Sanitary Pump Station and Forcemain

- one (1) 3000 mm diameter and 8.65 metre deep wet well with provision for three (3) submersible non-clog wastewater pumps, each pump designed for 42 litres/second at a Total Dynamic Head (TDH) of 29 metres, complete with trash basket, pipe rails, level regulation, force air blower unit and appurtenances;
- two (2) pumps will be initially installed with each pump capable of delivering 42 litres/second at a TDH of 29 metres for an initial firm capacity of 42 litres/second;
- the third pump will be added through an amendment to the ECA once development flows approach 42 litres/second to bring the pump station to its ultimate firm capacity of 84 litres/second;
- approximately 870 metres of dual 200 mm diameter HDPE DR13.5 sanitary forcemains originating at the pump station control building and terminating at the existing sanitary sewer on Fernbank Road;
- one (1) 2400 mm x 1800 mm concrete discharge manhole, complete with Swab Catcher, replacing the existing sanitary MH 401 on Fernbank Road. Dual forcemains will discharge to this new manhole;
- pump station control building complete with mechanical and electrical systems, process piping, valves, control panels, SCADA system, odour control system, swab launchers and appurtenances;

- one (1) 170 KW self-enclosed diesel generator (to be registered under Environmental Activity and Sector Registry (EASR)) on a reinforced concrete pad adjustment to the pump station control building complete with diesel fuel tank, valves and controls;
- one (1) 2400 mm x 1800 mm concrete by-pass chamber complete with valves, couplings and appurtenances;

Sanitary Sewers Pump Station

- approximately 4.8 metres of 450 mm diameter sanitary sewer @ 2.55% from Sanitary MH 99 to wet well;
- approximately 18 metres of 200 mm diameter sanitary forcemain HDPE 13.5 from SAN MH 99 to By-pass Chamber;
- approximately 18.7 metres of 600 mm diameter sanitary sewer from SAN MH 99 to SAN MH 97;

Interim Emergency Sanitary Sewer Overflow

approximately 26.6 metres of 250 mm diameter sanitary sewer from sanitary MH 97 to the existing Faulkner Ditch. Elevation of emergency overflow in sanitary MH 97 is 104.27m;

Permanent Emergency Sanitary Sewer Overflow

the permanent Emergency Sanitary Sewer Overflow will discharge to the future Davidson Stormwater Management Facility which is anticipated to be constructed within the next 2-4 years;

- the permanent emergency sanitary sewer overflow will consist of 3 metres of 600 mm diameter sewer from sanitary MH 97 to the future stormwater management facility. The elevation of the emergency overflow in MH 97 is 103.40m;
- provision to adjust the elevation of the permanent emergency sanitary overflow in MH 97 within a range of 102.80m to 103.70m based on the final 100-year water level in the future Davidson Stormwater Management Facility;
- one (1) primary measuring device in MH 97 consisting of a broad crest weir complete with ultrasonic level recorder;
- once permanent emergency sanitary sewer overflow is established, the interim overflow will be abandoned;

including erosion/sedimentation control measures during construction and all other controls and appurtenances essential for the proper operation of the aforementioned Works;

all in accordance with the application from the Stittsville South Inc. and 1384341 Ontario Ltd., dated March 03, 2016, and all other supporting documents, final plans and specifications prepared by Novatech.

For the purpose of this environmental compliance approval, the following definitions apply:

"Approval" means this entire document including the application and any supporting documents listed in any schedules in this Approval;

"BOD5" (also known as TBOD5) means five day biochemical oxygen demand measured in an unfiltered sample and includes carbonaceous and nitrogenous oxygen demand;

"Director" means a person appointed by the Minister pursuant to section 5 of the Environmental Protection Act for the purposes of Part II.1 of the Environmental Protection Act;

"E. Coli" refers to the thermally tolerant forms of Escherichia that can survive at 44.5 degrees Celsius;

"Emergency Situation" means a structural, mechanical or electrical failure that causes a temporary reduction in the capacity of the sanitary sewage pumping station or an unforeseen flow condition that may result in:

- a) danger to the health or safety of any person; or
- b) injury or damage to any property, or serious risk of injury or damage to any property.

"EPA" means the Environmental Protection Act, R.S.O. 1990, c.E.19, as amended;

"Event" in the context the sanitary sewage pumping station located outside a Sewage Treatment Plant, means an action or occurrence, at the sanitary sewage pumping station that causes a Sewage Pumping Station Overflow. An Event ends when there is no recurrence of a Sewage Pumping Station Overflow in the 12-hour period following the last Sewage Pumping Station Overflow. Two Events are separated by at least 12 hours during which there has been no recurrence of a Sewage Pumping Station Overflow;

"Limited Operational Flexibility" (LOF) means the modifications that the Owner is permitted to make to the Works under this Approval;

"Ministry" means the ministry of the government of Ontario responsible for the Environmental Protection Act and the Ontario Water Resources Act and includes all officials, employees or other persons acting on its behalf;

"Notice of Modifications" means the form entitled "Notice of Modifications to Sewage Works" included in Schedule "A";

"Owner" means the Stittsville South Inc. and 1384341 Ontario Ltd., and includes their successors and assignees;

"Professional Engineer" means a person entitled to practise as a Professional Engineer in the Province of Ontario under a licence issued under the Professional Engineers Act;

"Sewage Pumping Station Overflow" means any discharge from a sanitary sewage pumping station located outside a Sewage Treatment Plant that does not undergo any treatment or only receives partial treatment before it is discharged to the environment;

"Substantial Completion" has the same meaning as "substantial performance" in the Construction Lien Act;

"Water Supervisor" means the person appointed as Water Supervisor of the Ottawa office of the Ministry;

"Works" means the sewage works described in the Owner's application(s) and this Approval.

You are hereby notified that this environmental compliance approval is issued to you subject to the terms and conditions outlined below:

TERMS AND CONDITIONS

1. GENERAL PROVISIONS

(1) The Owner shall ensure that any person authorized to carry out work on or operate any aspect of the Works is notified of this Approval and the Conditions herein and shall take all reasonable measures to ensure any such person complies with the same.

(2) The designation of the The City of Ottawa as the operating authority of the site on the application for approval of the Works does not relieve the Owner from the responsibility of complying with any and all of the Conditions of this Approval.

(3) Except as otherwise provided by these Conditions, the Owner shall design, build, install, operate and maintain the Works in accordance with the description given in this Approval, and the application for approval of the Works.

(4) Where there is a conflict between a provision of any submitted document referred to in this Approval and the Conditions of this Approval, the Conditions in this Approval shall take precedence, and where there is a conflict between the listed submitted documents, the document bearing the most recent date shall prevail.

(5) Where there is a conflict between the listed submitted documents, and the application, the application shall take precedence unless it is clear that the purpose of the document was to amend the application.

(6) The Conditions of this Approval are severable. If any Condition of this Approval, or the application of any requirement of this Approval to any circumstance, is held invalid or unenforceable, the application of such Condition to other circumstances and the remainder of this Approval shall not be affected thereby.

(7) The issuance of, and compliance with the Conditions of this Approval does not:

(a) relieve any person of any obligation to comply with any provision of any applicable statute, regulation or other legal requirement, including, but not limited to, the obligation to obtain approval from the local conservation authority necessary to construct or operate the sewage Works; or

(b) limit in any way the authority of the Ministry to require certain steps be taken to require the

Owner to furnish any further information related to compliance with this Approval.

2. EXPIRY OF APPROVAL

(1) This Approval will cease to apply to those parts of the new Works which have not been constructed within **five (5) years** of the date of this Approval.

3. CHANGE OF OWNER

(1) The Owner shall notify the Director, in writing, of any of the following changes within **thirty (30) days** of the change occurring:

(a) change of Owner;

(b) change of address of the Owner;

(c) change of partners where the Owner is or at any time becomes a partnership, and a copy of the most recent declaration filed under the Business Names Act, R.S.O. 1990, c. B17 shall be included in the notification to the Director;

(d) change of name of the corporation where the Owner is or at any time becomes a corporation, and a copy of the most current information filed under the Corporations Information Act, R.S.O. 1990, c. C39 shall be included in the notification to the Director.

4. UPON SUBSTANTIAL COMPLETION OF THE SEWAGE PUMPING STATION

(1) Upon Substantial Completion of the sewage pumping station, the Owner shall prepare a statement, certified by a Professional Engineer, that the sewage pumping station was constructed in accordance with this Approval, and shall make the written statement available to the Ministry, upon request.

(2) Within **one (1) year** of Substantial Completion of the sewage pumping station, a set of as-built drawings showing the sewage pumping station “as constructed” shall be prepared. These drawings shall be kept up to date through revisions undertaken from time to time and a copy shall be retained at the sewage pumping station for the operational life of the sewage pumping station.

5. SEWAGE PUMPING STATION OVERFLOW

(1) Any Sewage Pumping Station Overflow is prohibited, except:

(a) in an Emergency Situation;

(b) where the Sewage Pumping Station Overflow is a direct and unavoidable result of a planned maintenance procedure, the Owner notified the Water Supervisor **fifteen (15) days** prior to the Sewage Pumping Station Overflow and the Water Supervisor has given written consent of the Sewage Pumping Station Overflow; or,

(c) where the Sewage Pumping Station Overflow is planned for research or training purposes, the discharger notified the Water Supervisor **fifteen (15) days** prior to the Sewage Pumping Station Overflow and the Water Supervisor has given written consent of the Sewage Pumping Station Overflow.

(2) The Owner shall forthwith notify the Spills Action Centre (SAC) at 1-800-268-6060 or e-mail at moe.sac.moe@ontario.ca and the Medical Officer of Health of every Sewage Pumping Station Overflow Event. This notice shall include, at a minimum, the following information:

- (a) the date and time at which the Event(s) started,
- (b) duration of the Event(s);
- (c) the location of the Event(s);
- (d) the measured or estimated volume of the Event(s) (unless the Event(s) is/are ongoing); and
- (e) the reason for the Event (s).

(3) The Owner shall submit Sewage Pumping Station Overflow Event Reports to the Ministry's local office on an Annual basis, no later than forty-five (45) days following the end of the calendar year. Event Reports shall be in an electronic format specified by the Ministry. In each Event Report the Owner shall include, at a minimum, the following information on any Event(s) that occurred during the preceding year:

- (a) the date and time at which the Event(s) started,
- (b) duration of the Event(s);
- (c) the location of the Event(s);
- (d) the measured or estimated volume of the Event(s) (unless the Event(s) is/are ongoing); and
- (e) the reason for the Event(s).

(4) The Owner shall use best efforts to collect a representative sample consisting of a minimum of two (2) grab samples of the Sewage Pumping Station Overflow and have it analysed for parameters outlined in Table 1 of Condition 7 (2) using the protocols specified in Condition 7 (3), one at the beginning of the Event and the second approximately near the end of the Event, to best reflect the effluent quality of such Sewage Pumping Station Overflow.

(5) The Owner shall maintain a record of all Sewage Pumping Station Overflow(s), which shall contain, at a minimum, the types of information set out in Condition 5 (2 (a)) to 5 (2 (e)) in respect of each Sewage Pumping Station Overflow.

6. OPERATION AND MAINTENANCE

(1) The Owner shall exercise due diligence in ensuring that, at all times, the Works and the related equipment and appurtenances used to achieve compliance with this Approval are properly operated and maintained. Proper operation and maintenance shall include effective performance, adequate funding, adequate operator staffing and training, including training in all procedures and other requirements of this Approval and the Act and regulations, adequate laboratory facilities, process controls and alarms and the use of process chemicals and other substances used in the Works.

(2) The Owner shall prepare an operations manual within **six (6) months** of Substantial Completion of the sewage pumping station, that includes, but not necessarily limited to, the following information:

(a) operating procedures for routine operation of the sewage pumping station;

(b) inspection programs, including frequency of inspection, for the sewage pumping station and the methods or tests employed to detect when maintenance is necessary;

(c) repair and maintenance programs, including the frequency of repair and maintenance for the sewage pumping station;

(d) procedures for the inspection and calibration of monitoring equipment;

(e) a spill prevention control and countermeasures plan, consisting of contingency plans and procedures for dealing with equipment breakdowns, potential spills and any other abnormal situations, including notification of the Water Supervisor; and

(f) procedures for receiving, responding and recording public complaints, including recording any follow-up actions taken.

(3) The Owner shall maintain the operations manual current and retain a copy at the location of the sewage pumping station for the operational life of the sewage pumping station. The Owner shall make the manual available to the Ministry, upon request.

(4) The Owner shall make all manuals, plans, records, data, procedures and supporting documentation available to the Ministry, upon request.

7. MONITORING AND RECORDING

The Owner shall, upon the issuance of this Approval, carry out the following monitoring program:

(1) All samples and measurements taken for the purposes of this Approval are to be taken at a time and in a location characteristic of the quality and quantity of the effluent stream over the time period being monitored.

(2) Samples shall be collected at the following sampling points, at the frequency specified, by means of

the specified sample type and analysed for each parameter listed and all results recorded:

Table 1 - Monitoring during a Sewage Pumping Station Overflow Event (Samples to be collected from the Sewage Pumping Station Overflow sewer near the sewage pumping station)	
Sample Type	Grab
Parameters	BOD5, Total Suspended Solids, Total Phosphorus, E. Coli (E. Coli samples may be limited to overflows occurring between Apr 1 and Oct 31)

(3) The methods and protocols for sampling, analysis and recording shall conform, in order of precedence, to the methods and protocols specified in the following:

(a) the Ministry's Procedure F-10-1, "Procedures for Sampling and Analysis Requirements for Municipal and Private Sewage Treatment Works (Liquid Waste Streams Only), as amended from time to time by more recently published editions;

(b) the Ministry's publication "Protocol for the Sampling and Analysis of Industrial/Municipal Wastewater" (January 1999), ISBN 0-7778-1880-9, as amended from time to time by more recently published editions; and

(c) the publication "Standard Methods for the Examination of Water and Wastewater" (21st edition), as amended from time to time by more recently published editions.

8. REPORTING

(1) **Fifteen (15) days** prior to the date of a planned Sewage Pumping Station Overflow being conducted pursuant to Condition 5 and as soon as possible for an unplanned Sewage Pumping Station Overflow, the Owner shall notify the Water Supervisor in writing of the pending start date, in addition to an assessment of the potential adverse effects on the environment and the duration of the Sewage Pumping Station Overflow.

(2) In addition to the obligations under Part X of the Environmental Protection Act, (which includes contacting the Spills Action Centre (SAC) at 1-800-268-6060 or e-mail at moe.sac.moe@ontario.ca), the Owner shall, within **ten (10) working days** of the occurrence of any reportable spill as defined in Ontario Regulation 675/98, Bypass or loss of any product, by-product, intermediate product, oil, solvent, waste material or any other polluting substance into the environment, (with the exception of a sanitary sewage discharged during an Event), submit a full written report of the occurrence to the Water Supervisor describing the cause and discovery of the spill or loss, clean-up and recovery measures taken, preventative measures to be taken and schedule of implementation.

(3) The Owner shall prepare and submit a report to the Water Supervisor on an annual basis. The reports shall contain the following information:

(a) a copy of all Notice of Modifications submitted to the Water Supervisor as a result of Schedule A, Section 1 (Limited Operational Flexibility) with a status report on the implementation of each modification;

(b) a report summarizing all modifications completed as a result of Schedule A, Section 3.

9. LIMITED OPERATIONAL FLEXIBILITY

(1) The Owner may make modifications to the Works in accordance with the Terms and Conditions of this Approval and subject to the Ministry's "Limited Operational Flexibility Criteria for Modifications to Sewage Works", included under Schedule "A" of this Approval, as amended.

(2) The sewage pumping station works proposed under Limited Operational Flexibility shall adhere to the design guidelines contained within the Ministry's publication "Design Guidelines for Sewage Works 2008", as amended.

(3) The Owner shall ensure at all times, that the sewage pumping station works, related equipment and appurtenances which are installed or used to achieve compliance are operated in accordance with all Terms and Conditions of this Approval.

(4) For greater certainty, the following are not permitted as part of Limited Operational Flexibility:

(a) Modifications to the sewage pumping station works that result in an increase of the Rated Capacity of the sewage pumping station works;

(b) Modifications to the sewage pumping station works that may adversely affect the approved effluent quality criteria or the location of the discharge/outfall;

(c) Modifications to the sewage pumping station works approved under s.9 of the EPA, and

(d) Modifications to the sewage pumping station works pursuant to an order issued by the Ministry.

(5) Implementation of Limited Operational Flexibility is not intended to be used for piecemeal measures that result in major alterations or expansions.

(6) If the implementation of Limited Operational Flexibility requires changes to be made to the Emergency Response, Spill Reporting and Contingency Plan, the Owner shall, as deemed necessary in consultation with the Water Supervisor, provide a revised copy of this plan for approval to the local fire services authority prior to implementing Limited Operational Flexibility.

(7) For greater certainty, any alteration made under the Limited Operational Flexibility may only be carried out after other legal obligations have been complied with including those arising from the Environmental Protection Act, Niagara Escarpment Planning and Development Act, Oak Ridges Moraine Conservation Act, Lake Simcoe Protection Act and Greenbelt Act.

(8) Prior to implementing Limited Operational Flexibility, the Owner shall complete a Notice of Modifications describing any proposed modifications to the sewage pumping station works and submit it to the Water Supervisor.

10. TEMPORARY EROSION AND SEDIMENT CONTROL

(1) The Owner shall install and maintain temporary sediment and erosion control measures during construction and conduct inspections once every **two (2) weeks** and after each significant storm event (a significant storm event is defined as a minimum of 25 mm of rain in any 24 hours period). The inspections and maintenance of the temporary sediment and erosion control measures shall continue until they are no longer required and at which time they shall be removed and all disturbed areas reinstated properly.

(2) The Owner shall maintain records of inspections and maintenance which shall be made available for inspection by the Ministry, upon request. The record shall include the name of the inspector, date of inspection, and the remedial measures, if any, undertaken to maintain the temporary sediment and erosion control measures.

11. RECORD KEEPING

The Owner shall retain for a minimum of **five (5) years** from the date of their creation, all records and information related to or resulting from the operation and maintenance activities required by this Approval.

Schedule "A"

Limited Operational Flexibility Criteria for Modifications to Sewage Works

1. The modifications to a sewage pumping station approved under an Environmental Compliance Approval (Approval) that are permitted under the Limited Operational Flexibility (LOF), are outlined below and are subject to the LOF conditions in the Approval, and require the submission of the Notice of Modifications. If there is a conflict between the sewage pumping station works listed below and the Terms and Conditions in the Approval, the Terms and Conditions in the Approval shall take precedence.
 - 1.1 Sewage Pumping Stations
 - a. Adding or replacing equipment where new equipment is located within an existing sewage pumping station site, provided that the facility Rated Capacity is not exceeded and the existing flow process and/or treatment train are maintained, as applicable.
 - 1.2 Pilot Systems
 - a. Installation of pilot systems for new or existing technologies provided that:
 - i. any effluent from the pilot system is discharged to the inlet of the sewage pumping station or hauled off-site for proper disposal,
 - ii. any effluent from the pilot system discharged to the inlet of the sewage pumping station or sewage conveyance system does not significantly alter the composition/concentration of the influent sewage to be treated in the downstream process; and that it does not add any inhibiting substances to the downstream process, and
 - iii. the pilot system's duration does not exceed a maximum of two years; and a report with results is submitted to the Director and Water Supervisor three months after completion of the pilot project.
2. Sewage works that are exempt from section 53 of the OWRA by O. Reg. 525/98 continue to be exempt and are not required to follow the notification process under this Limited Operational Flexibility.
3. Normal or emergency operational modifications, such as repairs, reconstructions, or other improvements that are part of maintenance activities, including cleaning, renovations to existing approved sewage works equipment, provided that the modification is made with Equivalent Equipment, are considered pre-approved.
4. The modifications noted in section (3) above are not required to follow the notification protocols under Limited Operational Flexibility, provided that the number of pieces and description of the equipment as described in the Approval does not change.

Notice of Modification to Sewage Works

RETAIN COPY OF COMPLETED FORM AS PART OF THE ECA AND SEND A COPY TO THE WATER SUPERVISOR (FOR MUNICIPAL) OR DISTRICT MANAGER (FOR NON-MUNICIPAL SYSTEMS)

Part 1 – Environmental Compliance Approval (ECA) with Limited Operational Flexibility

(Insert the ECA's owner, number, issuance date and notice number, which should start with "01" and consecutive numbers thereafter)

ECA Number	Issuance Date (mm/dd/yy)	Notice number (if applicable)
ECA Owner		Municipality

Part 2: Description of the modifications as part of the Limited Operational Flexibility

(Attach a detailed description of the sewage works)

Description shall include:

1. A detail description of the modifications and/or operations to the sewage works (e.g. sewage work component, location, size, equipment type/model, material, process name, etc.)
2. Confirmation that the anticipated environmental effects are negligible.
3. List of updated versions of, or amendments to, all relevant technical documents that are affected by the modifications as applicable, i.e. submission of documentation is not required, but the listing of updated documents is (design brief, drawings, emergency plan, etc.)

Part 3 – Declaration by Professional Engineer

I hereby declare that I have verified the scope and technical aspects of this modification and confirm that the design:

1. Has been prepared or reviewed by a Professional Engineer who is licensed to practice in the Province of Ontario;
 2. Conforms with the Limited Operational Flexibility as per the ECA;
 3. Has been designed consistent with Ministry's Design Guidelines, adhering to engineering standards, industry's best management practices, and demonstrating ongoing compliance with s.53 of the Ontario Water Resources Act; and other appropriate regulations.
- I hereby declare that to the best of my knowledge, information and belief the information contained in this form is complete and accurate.

Name (Print)	PEO License Number
Signature	Date (mm/dd/yy)
Name of Employer	

Part 4 – Declaration by Owner

I hereby declare that:

1. I am authorized by the Owner to complete this Declaration;

2. The Owner consents to the modification; and
 3. These modifications to the sewage works are proposed in accordance with the Limited Operational Flexibility as described in the ECA.
 4. The Owner has fulfilled all applicable requirements of the *Environmental Assessment Act*.
 I hereby declare that to the best of my knowledge, information and belief the information contained in this form is complete and accurate.

Name of Owner Representative (Print)	Owner representative's title (Print)
Owner Representative's Signature	Date (mm/dd/yy)

The reasons for the imposition of these terms and conditions are as follows:

- Condition 1 is imposed to ensure that the Works are built and operated in the manner in which they were described for review and upon which approval was granted. This Condition is also included to emphasize the precedence of Conditions in the Approval and the practice that the Approval is based on the most current document, if several conflicting documents are submitted for review.
- Condition 2 is included to ensure that, when the Works are constructed, the Works will meet the standards that apply at the time of construction to ensure the ongoing protection of the environment.
- Condition 3 is included to ensure that the Ministry records are kept accurate and current with respect to approved Works and to ensure that any subsequent Owner of the Works is made aware of the Approval and continue to operate the Works in compliance with it.
- Condition 4 is included to ensure that the sewage pumping station is constructed in accordance with the Approval and that record drawings of the sewage pumping station "as constructed" are maintained for future reference.
- Conditions 5 and 7 are included to indicate that Sewage Pumping Station Overflow of untreated and/or partially treated sewage to the environment is prohibited, save in certain limited circumstances where the failure to do so could result in greater injury to the public interest than the Sewage Pumping Station Overflow itself, or where the Sewage Pumping Station Overflow can be limited or otherwise mitigated by handling it in accordance with an approved contingency plan. The notification and documentation requirements allow the Ministry to take action in an informed manner and will ensure the Owner is aware of the extent and frequency of Sewage Pumping Station Overflow Event(s).
- Condition 6 is included to require that the Works be properly operated, maintained, funded, staffed and equipped such that the environment is protected and deterioration, loss, injury or damage to any person or property is prevented. As well, the inclusion of a comprehensive operations manual for the sewage pumping station governing all significant areas of operation, maintenance and repair is prepared, implemented and kept up-to-date by the Owner and made available to the Ministry. Such a manual is an integral part of the operation of the sewage pumping station. Its compilation and use should assist the Owner in staff training, in proper plant operation and in identifying and planning for contingencies during possible abnormal conditions. The manual will also act as a benchmark for Ministry staff when reviewing the Owner's operation of the Works.

7. Condition 8 is included to provide a performance record for future references, to ensure that the Ministry is made aware of problems as they arise, so that the Ministry can work with the Owner in resolving any problems in a timely manner.
8. Condition 9 is included to ensure that the Works are operated in accordance with the application and supporting documentation submitted by the Owner, and not in a manner which the Director has not been asked to consider. These Conditions are also included to ensure that a Professional Engineer has reviewed the proposed Modifications and attests that the Modifications are in line with that of Limited Operational Flexibility, and provide assurance that the proposed Modifications comply with the Ministry's requirements stipulated in the Terms and Conditions of this Approval, Ministry policies, guidelines, and industry engineering standards and best management practices.
9. Condition 10 is included as installation, regular inspection and maintenance of the temporary sediment and erosion control measures is required to mitigate the impact on the downstream receiving watercourse during construction, until they are no longer required.
10. Condition 11 is included to require that all records are retained for a sufficient time period to adequately evaluate the long-term operation and maintenance of the Works.

In accordance with Section 139 of the Environmental Protection Act, you may by written Notice served upon me and the Environmental Review Tribunal within 15 days after receipt of this Notice, require a hearing by the Tribunal. Section 142 of the Environmental Protection Act provides that the Notice requiring the hearing shall state:

1. The portions of the environmental compliance approval or each term or condition in the environmental compliance approval in respect of which the hearing is required, and;
2. The grounds on which you intend to rely at the hearing in relation to each portion appealed.

The Notice should also include:

3. The name of the appellant;
4. The address of the appellant;
5. The environmental compliance approval number;
6. The date of the environmental compliance approval;
7. The name of the Director, and;
8. The municipality or municipalities within which the project is to be engaged in.

And the Notice should be signed and dated by the appellant.

This Notice must be served upon:

The Secretary*
Environmental Review Tribunal
655 Bay Street, Suite 1500
Toronto, Ontario
M5G 1E5

AND

The Director appointed for the purposes of
Part II.1 of the Environmental Protection Act
Ministry of the Environment and
Climate Change
135 St. Clair Avenue West, 1st Floor
Toronto, Ontario
M4V 1P5

* Further information on the Environmental Review Tribunal's requirements for an appeal can be obtained directly from the Tribunal at: Tel: (416) 212-6349, Fax: (416) 326-5370 or www.ert.gov.on.ca

The above noted activity is approved under s.20.3 of Part II.1 of the Environmental Protection Act.

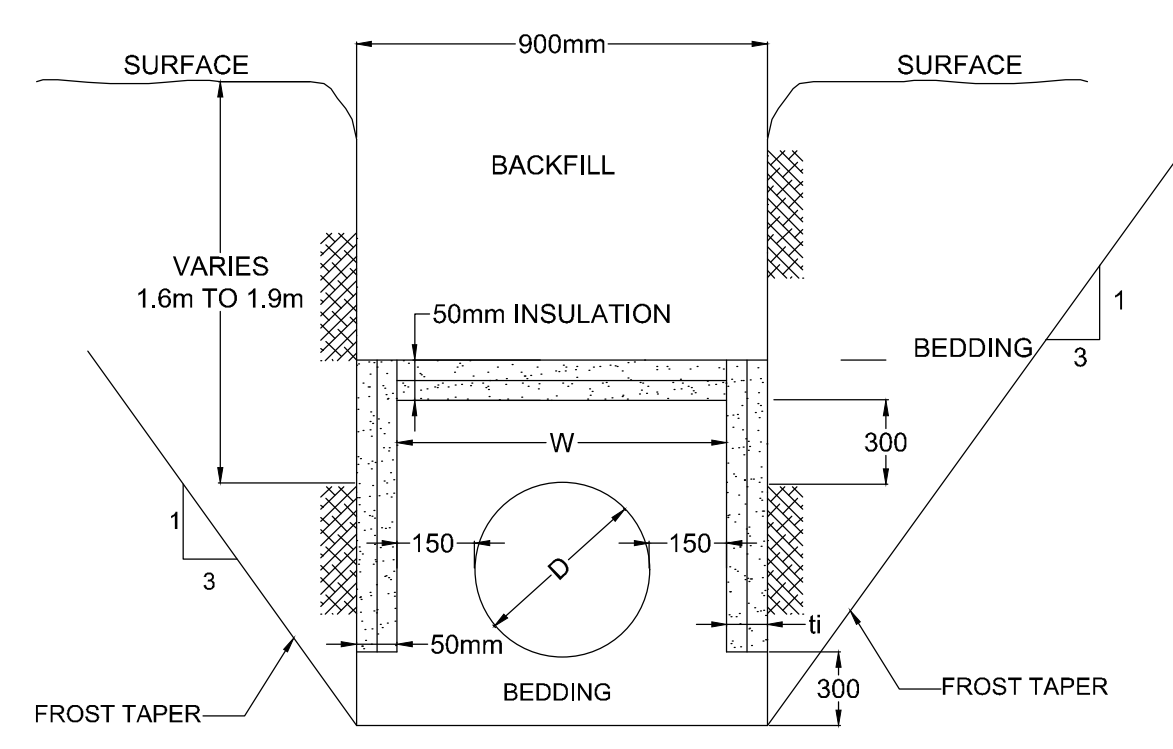
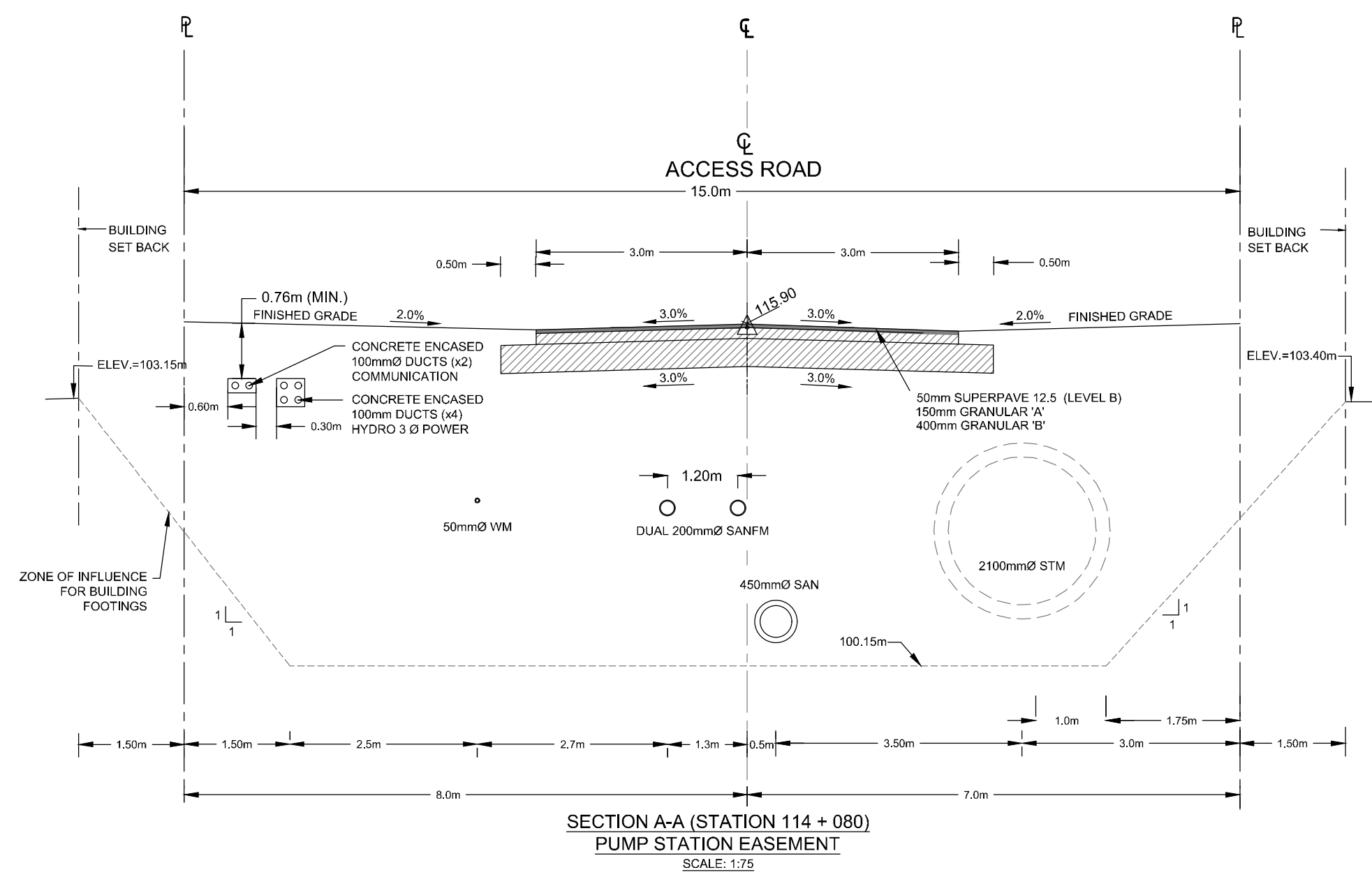
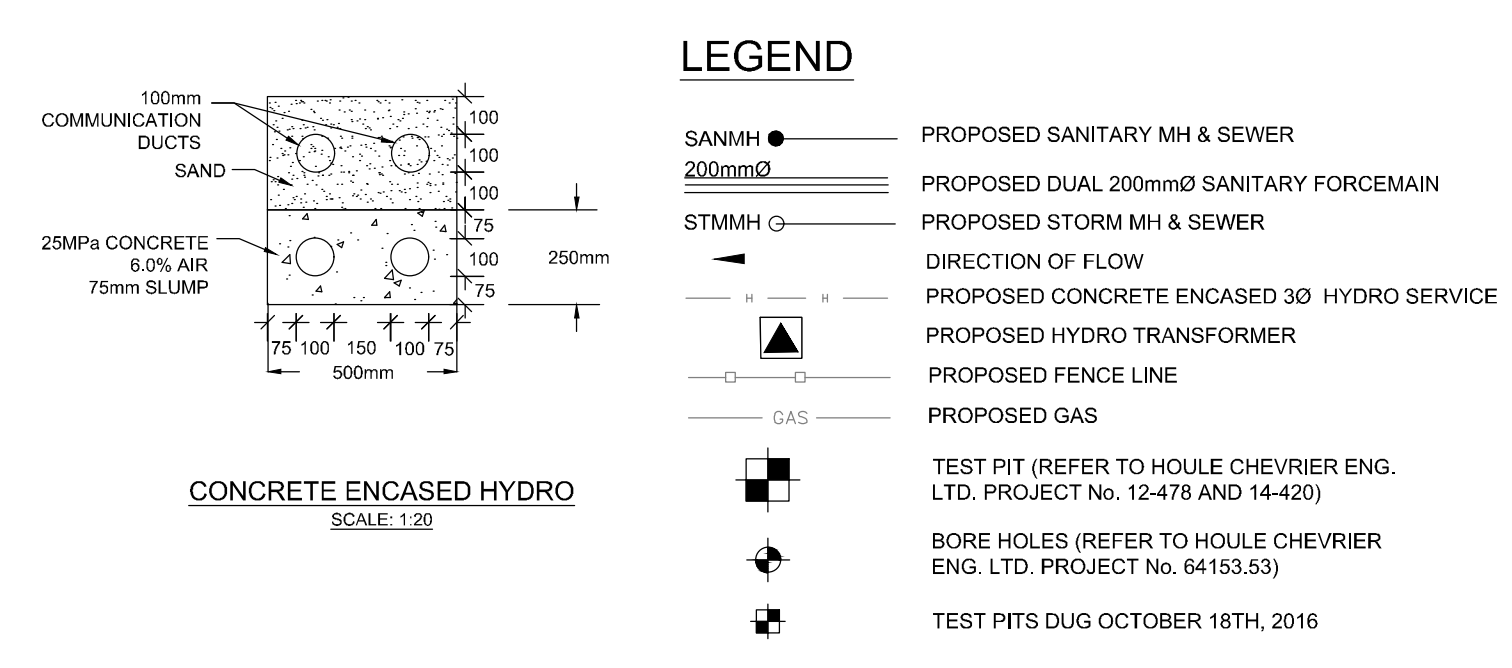
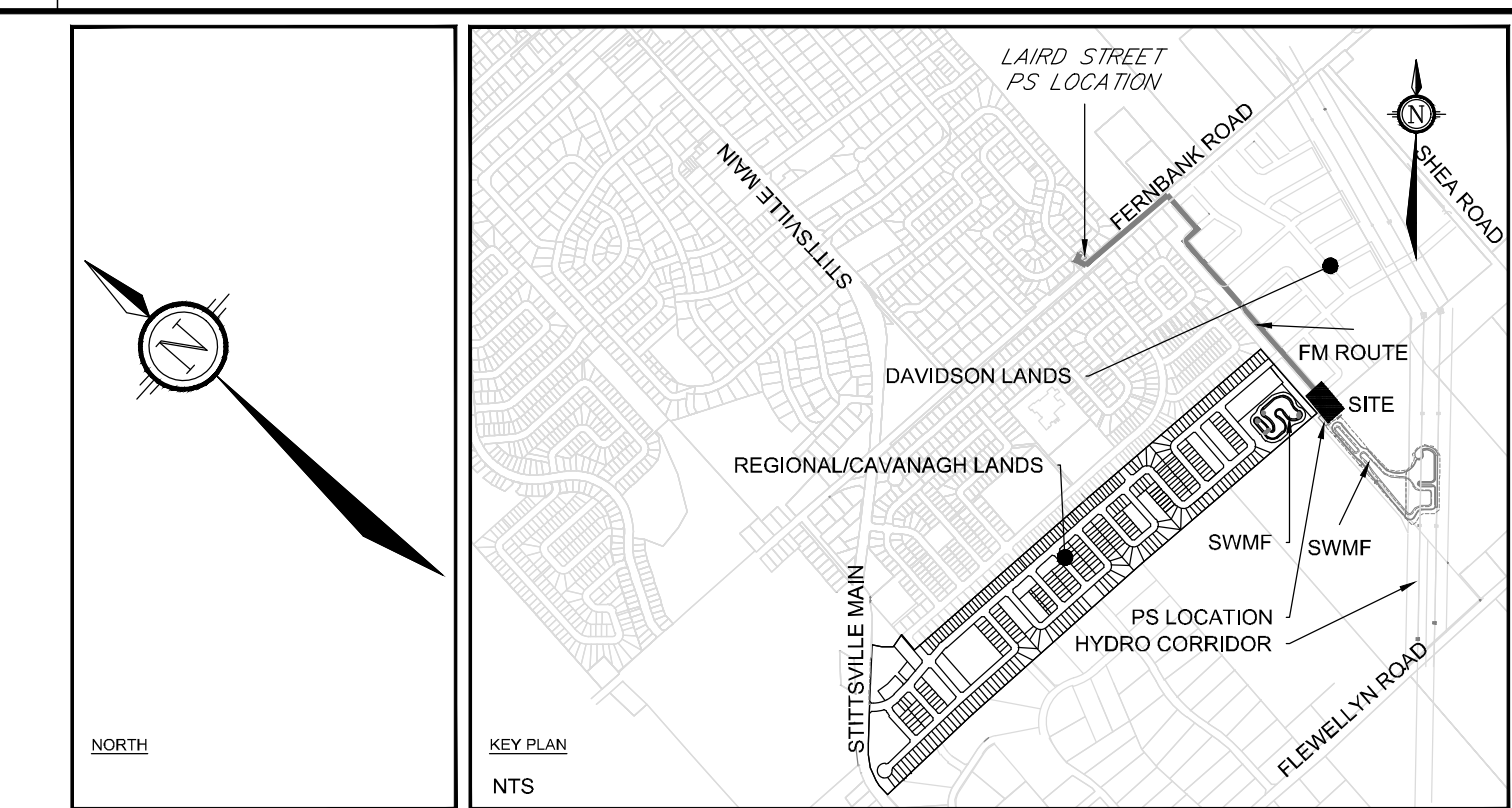
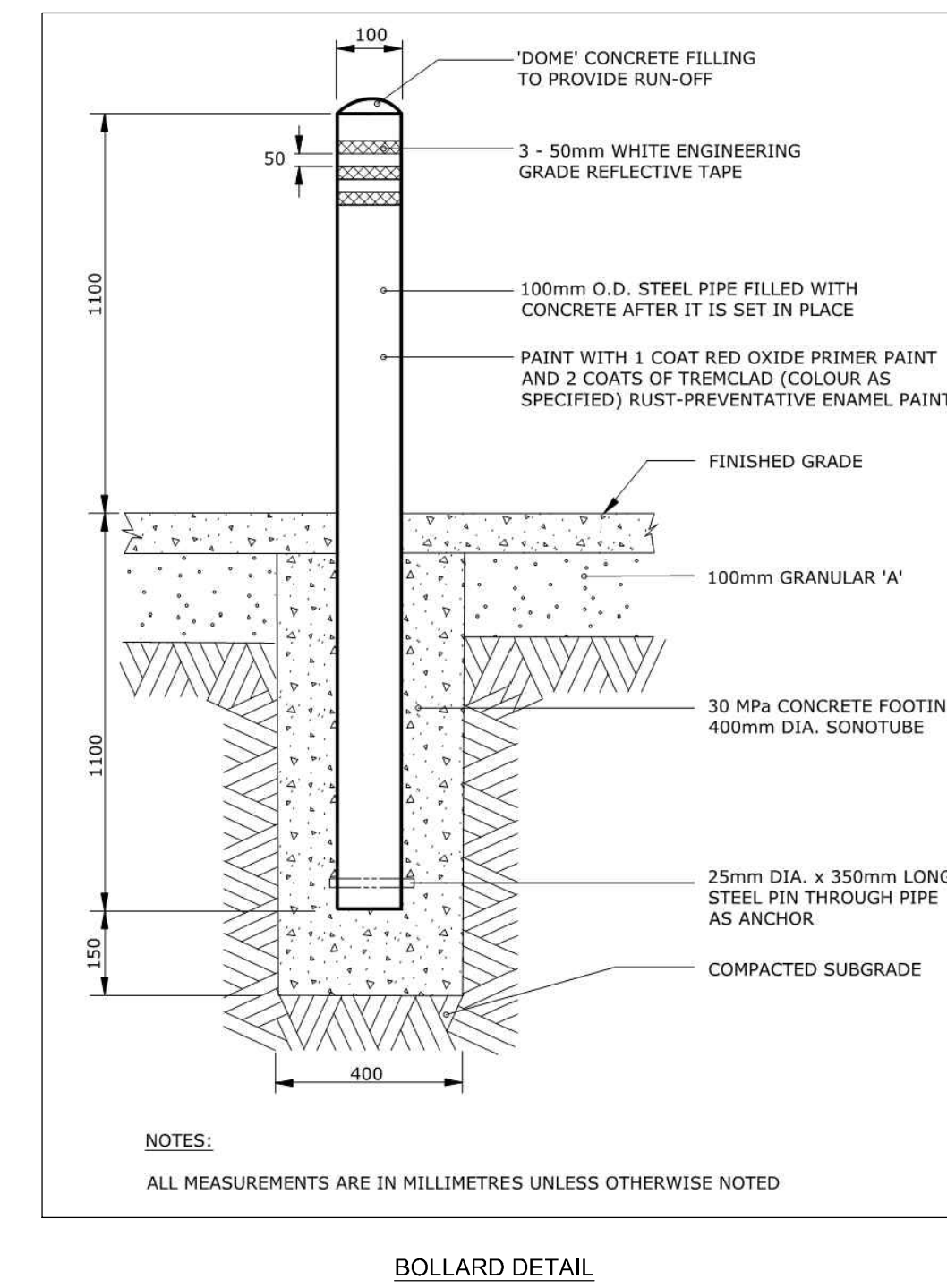
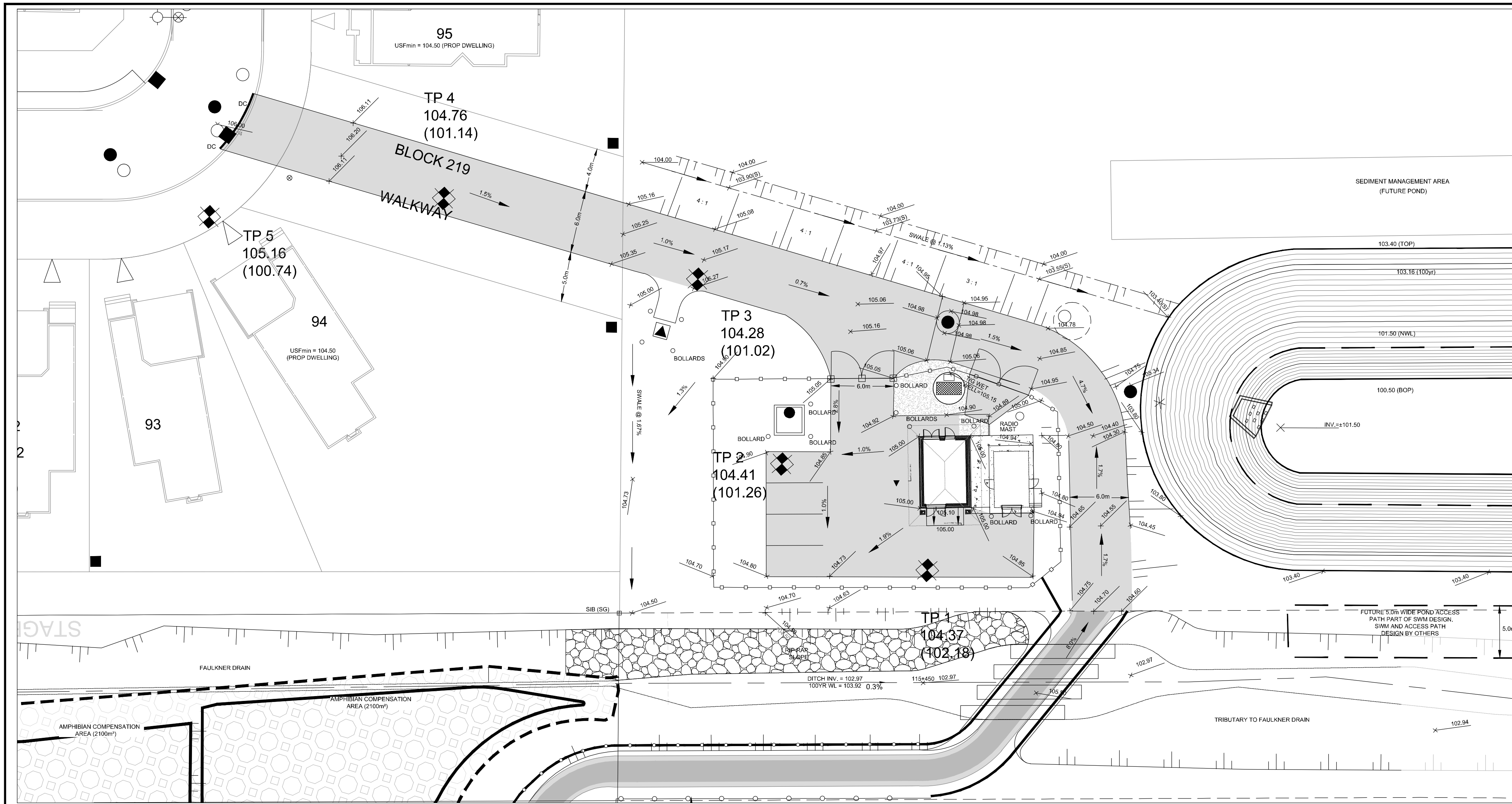
DATED AT TORONTO this 21st day of September, 2016



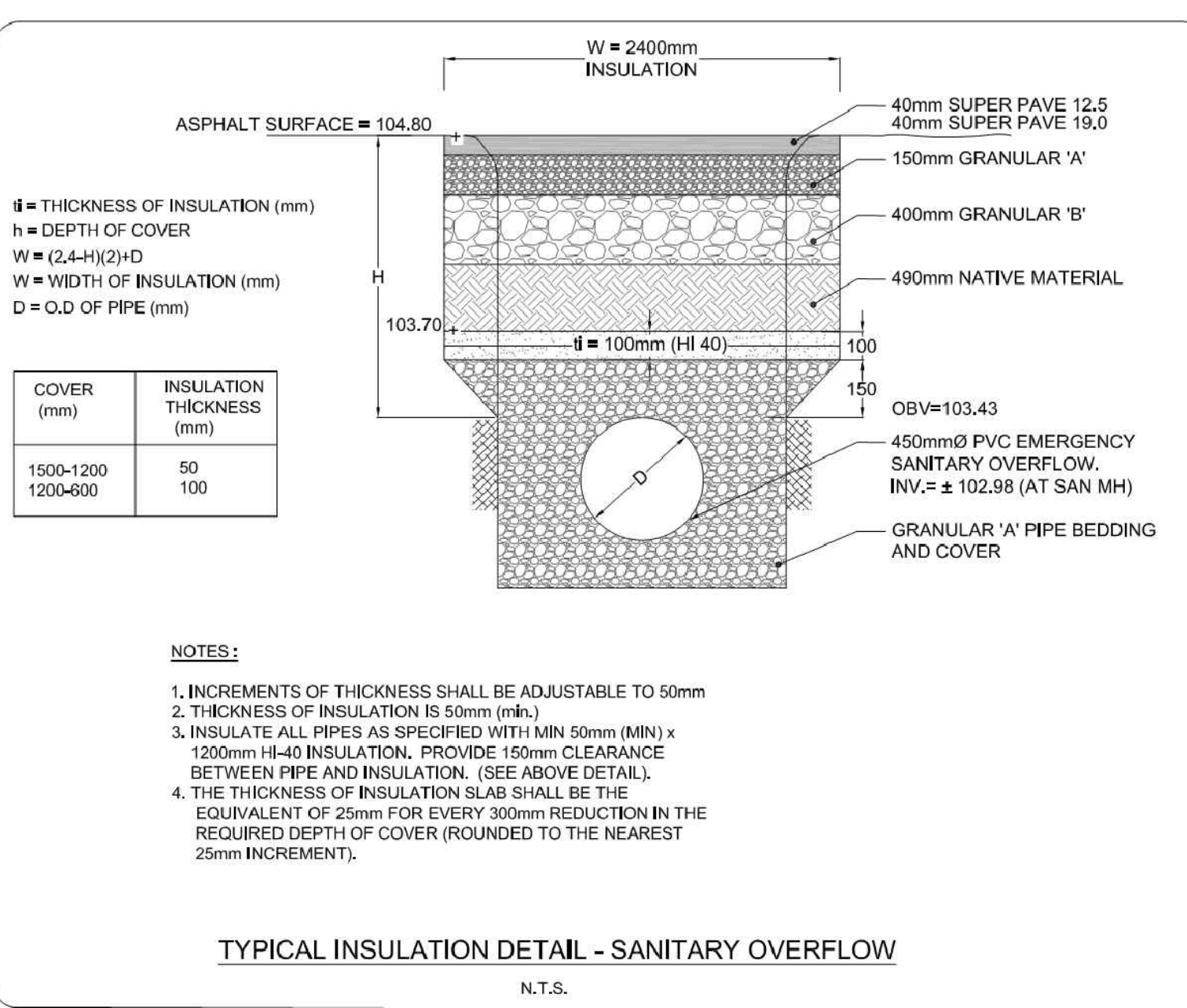
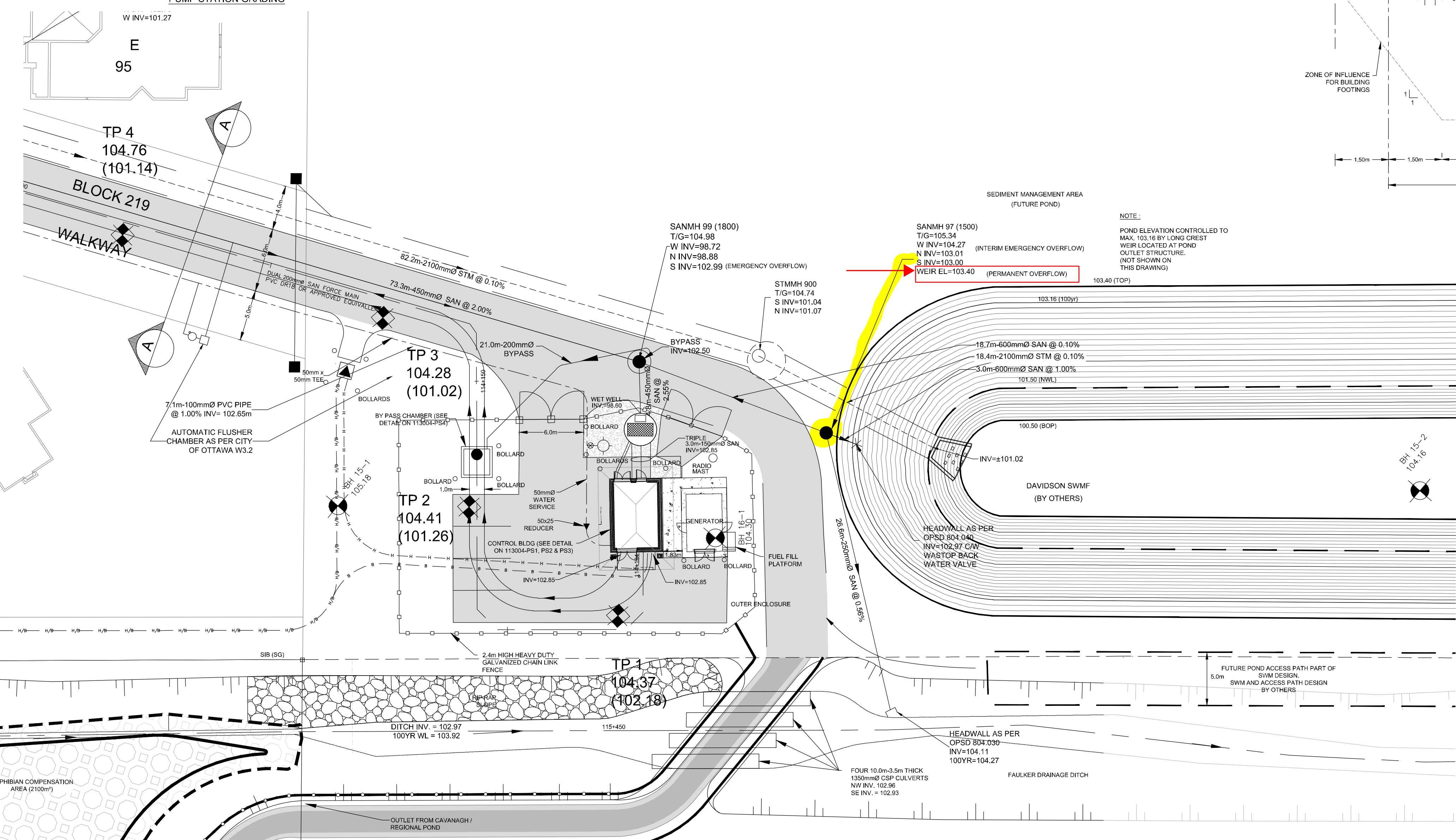
Gregory Zimmer, P.Eng.
Director
appointed for the purposes of Part II.1 of the
Environmental Protection Act

MS/

c: District Manager, MOECC Ottawa office
Greg McDonald, Novatech
Charles Warnock, Program Manager, City of Ottawa, Development Review
Linda Carkner, Program Manager, City of Ottawa, Infrastructure Services



SANITARY FORCEMAIN INSULATION DETAIL STA 114+110 TO 114+156 N.T.S.



NOTE: THE POSITION OF ALL POLE LINES, CONDUITS, WATERMANS, SEWERS AND OTHER UNDERGROUND AND OVERGROUND UTILITIES AND STRUCTURES IS NOT NECESSARILY SHOWN ON THE CONTRACT DRAWINGS, AND WHERE SHOWN, THE ACCURACY OF THE POSITION OF SUCH UTILITIES AND STRUCTURES IS NOT GUARANTEED. BEFORE STARTING WORK, DETERMINE THE EXACT LOCATION OF ALL SUCH UTILITIES AND STRUCTURES AND ASSUME ALL LIABILITY FOR DAMAGE TO THEM.

No.	REVISION	DATE	BY	No.	REVISION	DATE	BY
14.	AS BUILT	SEP 06/18	GJM	6.	ADDENDUM No. 4	JUL 05/16	GJM
13.	REVISED GRADES FROM SANMH 99 TO FAULKNER DRAIN	JUN 23/17	GJM	7.	REVISED MANHOLES 97 AND 99. 200mm BYPASS ADDED	JUN 22/16	GJM
12.	ISSUED FOR MYLARS	FEB 23/17	GJM	8.	ISSUED FOR TENDER	MAY 25/16	GJM
11.	ISSUED FOR LAYOUT	JAN 13/17	GJM	9.	ISSUED FOR ENVIRONMENTAL COMPLIANCE APPROVAL	MAY 04/16	GJM
10.	REVISED SANMH 99 AND 101 INVERTS	NOV 17/16	GJM	10.	ISSUED WITH MINOR ADJUSTMENTS	MAR 29/16	GJM
9.	AUTOMATIC FLUSHER CHAMBER ADDED	SEPT 09/16	BHB	11.	ISSUED FOR 75% DESIGN	MAR 14/16	GJM
				12.	50 % DESIGN	DEC 22/15	GJM
				13.	ISSUED WITH PUMP STATION PRE-DESIGN REPORT	JUNE 24/15	GJM

SCALE
1 : 250
(UNLESS NOTED)

DESIGN
GJM/BHB

CHECKED
GJM

DRAWN
MWC

APPROVED
GJM

DATE
GJM



LOCATION
CITY OF OTTAWA
STITTSVILLE SOUTH - AREA 6
DRAWING NAME
SHEA ROAD SANITARY PUMP STATION - SITE SERVICING AND GRADING
PROJECT No.
113004-00
REV # 14
DRAWING No.
113004-PS-SVC



IBI GROUP
400-333 Preston Street
Ottawa, Ontario K1S 5N4 Canada
tel 613 225 1311 fax 613 225 9868
ibigroup.com

SANITARY SEWER DESIGN SHEET

Davidson Lands
City of Ottawa
Name of Client/Developer

LOCATION				AREA w/ Units (Ha)	RESIDENTIAL				AREA w/o Units (Ha)	ICI AREAS				PEAK FLOW (L/s)	INFILTRATION ALLOWANCE			FIXED FLOW (L/s)	TOTAL FLOW (L/s)	PROPOSED SEWER DESIGN					
STREET	AREA ID	FROM MH	TO MH		SF	SD	TH	APT		IND	CUM	PEAK FACTOR	AREA (Ha)		FLOW (L/s)	CAPACITY (L/s)	LENGTH (m)			DIA (mm)	SLOPE (%)	VELOCITY (full) (m/s)	AVAILABLE CAPACITY		
								INSTITUTIONAL		COMMERCIAL		INDUSTRIAL		AREA (Ha)									L/s	%	
								IND	CUM	IND	CUM	IND	CUM	IND	CUM	IND	CUM								
PHASE 2																									
Block 170 186A MH186A MH187A 0.10 2.08 137.3 137.3 4.00 2.22 2.08 2.08 0.58 2.81 21.64 12.44 200 0.40 0.667 18.83 87.03%																									
Crosanti Drive 300A MH300A MH301A 0.20 4 10.8 10.8 4.00 0.18 0.2 0.20 0.06 0.23 27.56 11.47 200 0.65 0.850 27.33 99.16%																									
Maverick Crescent 301A MH301A MH302A 1.01 31 83.7 94.5 4.00 1.53 1.01 1.21 0.34 1.87 20.24 116.48 200 0.35 0.624 18.37 90.76%																									
302A MH302A MH303A 0.07 2 5.4 99.9 4.00 1.62 0.07 1.28 0.36 1.98 20.24 11.48 200 0.35 0.624 18.27 90.23%																									
303A MH303A MH304A 0.35 9 24.3 124.2 4.00 2.01 0.35 1.63 0.46 2.47 20.24 61.29 200 0.35 0.624 17.77 87.80%																									
304A MH304A MH305A 0.01 9 0.0 124.2 4.00 2.01 0.01 1.64 0.46 2.47 20.24 14.00 200 0.35 0.624 17.77 87.79%																									
Maverick Crescent 300A1 MH300A MH306A 0.75 26 70.2 70.2 4.00 1.14 0.75 0.75 0.21 1.35 27.59 92.03 200 0.65 0.851 26.24 95.12%																									
306A MH306A MH307A 0.08 0.08 70.2 4.00 1.14 0.08 0.83 0.23 1.37 20.24 12.96 200 0.35 0.624 18.87 93.23%																									
Ocala Street 307A MH307A MH308A 0.24 5 13.5 83.7 4.00 1.36 0.24 1.07 0.30 1.66 20.24 68.00 200 0.35 0.624 18.59 91.82%																									
308A MH308A MH305A 0.26 7 18.9 102.6 4.00 1.66 0.26 1.33 0.37 2.03 20.24 59.95 200 0.35 0.624 18.21 89.95%																									
Ocala Street 305A MH305A MH187A 0.06 0.06 226.8 4.00 3.68 0.06 3.03 0.85 4.52 20.24 50.21 200 0.35 0.624 15.72 77.65%																									
Crosanti Drive 187A MH187A MH188A 0.19 0.19 364.1 4.00 5.90 0.19 5.40 1.51 7.41 20.24 89.74 200 0.35 0.624 12.83 63.39%																									
188A MH188A MH189A 0.80 27 72.9 437.0 4.00 7.08 0.8 6.20 1.74 8.82 20.24 103.64 200 0.35 0.624 11.43 56.45%																									
189A MH189A CAP 0.12 2 5.4 442.4 4.00 7.17 0.12 6.32 1.77 8.94 21.64 7.33 200 0.40 0.667 12.70 58.70%																									
189A CAP MH108A 0.0 0.0 442.4 4.00 7.17 0 6.32 1.77 8.94 21.64 16.00 200 0.40 0.667 12.70 58.70%																									
PHASE 1																									
Edenwyde Drive 108A MH108A MH109A 0.26 5 13.5 145.1 3.69 21.74 0.26 23.26 6.51 30.63 50.44 75.91 300 0.25 0.691 19.81 39.28%																									
PHASE 2																									
Orvieto Way 211A MH211A CAP210AE 0.65 13 35.1 35.1 4.00 0.57 0.65 0.65 0.18 0.75 27.59 86.14 200 0.65 0.851 26.84 97.28%																									
CAP210AE MH210A 0.0 35.1 4.00 0.57 0 0.65 0.18 0.75 27.59 4.00 200 0.65 0.851 26.84 97.28%																									
MH210A MH109A 0.0 35.1 4.00 0.57 0 0.65 0.18 0.75 27.59 11.31 200 0.65 0.851 26.84 97.28%																									
PHASE 1																									
Hickstead Way 109A MH109A MH110A 0.32 5 17.0 1507.2 3.68 22.46 0.32 24.23 6.78 31.61 50.44 76.01 300 0.25 0.691 18.83 37.33%																									
PHASE 2																									
FRIENDLY CRESCENT EXTERNAL EXMH181 MH230A 4.66 70 238.0 238.0 4.00 3.86 4.66 4.66 1.30 5.16 49.58 10.88 200 2.10 1.529 44.42 89.59%																									
Block 169 MH230A MH231A 0.0 238.0 4.00 3.86 0 4.66 1.30 5.16 49.58 45.03 200 2.10 1.529 44.42 89.59%																									
Sendero Way 231A MH231A MH221A 0.59 10 34.0 272.0 4.00 4.41 0.59 5.25 1.47 5.88 34.22 67.09 200 1.00 1.055 28.34 82.82%																									
221A MH221A MH222A 0.64 15 51.0 323.0 4.00 5.23 0.64 5.89 1.65 6.88 26.50 80.39 200 0.60 0.817 19.62 74.03%																									
222A MH222A MH223A 0.17 2 6.8 329.8 4.00 5.34 0.17 6.06 1.70 7.04 34.22 11.46 200 1.00 1.055 27.18 79.42%																									
223A MH223A MH224A 0.40 8 27.2 357.0 4.00 5.78 0.4 6.46 1.81 7.59 21.64 69.88 200 0.40 0.667 14.05 64.91%																									
Sendero Way 235A MH235A MH232A 0.56 8 27.2 27.2 4.00 0.44 0.56 0.56 0.16 0.60 34.22 61.94 200 1.00 1.055 33.62 98.25%																									
232A MH232A MH233A 0.13 1 3.4 30.6 4.00 0.50 0.13 0.69 0.19 0.69 48.39 11.34 200 2.00 1.492 47.70 98.58%																									
233A MH233A MH234A 0.45 10 34.0 64.6 4.00 1.05 0.45 1.14 0.32 1.37 21.64 57.60 200 0.40 0.667 20.27 93.69%																									
234A MH234A MH224A 0.59 13 44.2 108.8 4.00 1.76 0.59 1.73 0.48 2.25 21.64 83.39 200 0.40 0.667 19.39 89.61%																									
Sendero Way 224A MH224A CAP 225AN 0.17 3 10.2 476.0 3.99 7.68 0.17 8.36 2.34 10.03 26.50 37.03 200 0.60 0.817 16.48 62.18%																									
CAP 225AN MH225A 0.0 476.0 3.99 7.68 0 8.36 2.34 10.03 26.50 6.50 200 0.60 0.817 16.48 62.18%																									
225A MH225A MH226A 0.03 0.03 476.0 3.99 7.68 0.03 8.39 2.35 10.03 26.50 15.56 200 0.60 0.817 16.47 62.14%																									
226A MH226A MH110A 0.02 0.02 476.0 3.99 7.68 0.02 8.41 2.35 10.04 26.50 21.49 200 0.60 0.817 16.46 62.12%																									
Painted Sky Way 215A MH215A MH216A 0.62 14 47.6 47.6 4.00 0.77 0.62 0.62 0.17 0.94 24.19 83.61 200 0.50 0.746 23.25 96.09%																									
216A MH216A CAP 110AE 0.75 18 61.2 108.8 4.00 1.76 0.75 1.37 0.38 2.15 49.58 84.93 200 2.10 1.529 47.44 95.67%																									
CAP 110AE MH110A 0.0 108.8 4.00 1.76 0 1.37 0.38 2.15 49.58 15.00 200 2.10 1.529 47.44 95.67%																									
Hickstead Way 110A MH110A MH111A 0.36 6 20.4 2112.4 3.57 30.52 0.36 34.37 9.62 42.52 50.44 78.00 300 0.25 0.691 7.92 15.71%																									
Hickstead Way 176A MH176A MH111A 0.36 6 20.4 20.4 4.00 0.33 0.36 0.36 0.10 0.43 34.22 64.00 200 1.00 1.055 33.79 98.74%																									
Aridus Crescent 111A MH111A MH112A 0.47 10 34.0 2166.8 3.56 31.23 0.47 35.20 9.86 43.46 91.46 68.97 375 0.25 0.802 48.00 52.48%																									
112A MH112A MH113A 0.51 10 34.0 2200.8 3.55 31.68 0.51 35.71 10.00 44.05 91.46 66.19 375 0.25 0.802 47.41 51.94%																									
113A MH113A MH114A 0.12 1 3.4 2204.2 3.55 31.72 0.12 35.83 10.03 44.12 91.46 13.49 375 0.25 0.802 47.33 51.75%																									
114A MH114A 0.52 9 30.6 2234.8 3.55 32.12 0.52 36.35 10.18 44.67 91.46 55.98 375 0.25 0.802 46.79 51.16%																									
Structure - (325) MH148A 12.00 200 2.77 35.00 250 0.54																									
EXMH181 EXMH181 16.74 250 0.54																									
MH189A Structure - (328) 7.33 200 0.40																									
WORKS DESIGNED BY NOVATECH																									
STITTSVILLE SOUTH EXTERNAL EXMH181 MH189A 111 109 (0) 0.23 3 2182.0 2182.0 3.55 31.57 32.94 32.94 9.29 40.85 141.68 115.7 375 0.60 1.243 100.83 71.17%																									
175A 10.2 2192.2 3.55 31.57 0.23 33.17 9.29 40.85 141.68 115.7 375 0.60 1.243 100.83 71.17%																									
STREET NO. 11 176B 109 (0) 107 (0) 0.54 11 37.4 2229.6 3.55 32.05 0.54 33.71 9.44 41.49 91.46 71.7 375 0.25 0.802 49.97 54.63%																									
STREET NO. 11 177A 107 (0) 105 (0) 0.49 10 34.0 2263.6 3.54 32.49 0.49 34.20 9.58 42.07 91.46 62.1 375 0.25 0.802 49.39 54.00%																									
STREET NO. 11 178A 105 (0) MH115A 0.19 1 3.4 2267.0 3.54 32.54 0.19 34.39 9.63 42.16 258.68 11.0 375 2.00 2.269 216.51 83.70%																									
BLOCK 263 MH115A 99 (1800) 0.0 4501.8 3.29 59.94 2.73 0 70.74 19.81 82.12 320.35 73.3 450 1.16 1.951 238.23 74.37%																									
BLOCK 263 99 (1800) PS 0.0 4501.8 3.29 59.94 2.73 0 70.74 19.81 82.12 474.96 4.8 450 2.55 2.893 392.84 82.71%																									
Design Parameters:				Notes:				Designed:				Revision				Date									
Residential		ICI Areas		Peak Factor		1. Mannings coefficient (n) = 0.013		350 L/day		LME		1. City submission No. 1		2019-10-30											
SF 3.4 p/p/u				1.5		2. Demand (per capita): 350 L/day				Checked:		2. City submission No. 2		2020-02-07											
TH/SD 2.7 p/p/u		INST 50,000 L/ha/day		1.5		3. Infiltration allowance: 0.28 L/s/ha				Dwg. Reference: 37533-501		3. City submission No. 3		2020-04-09											
APT 1.8 p/p/u		COM 50,000 L/ha/day		1.5		4. Residential Peaking Factor: Harmon Formula = 1+(14/(4+P^0.5))				File Reference: 37533-5.7.1		Date: 2020-04-08		Sheet No: 2 of 2											
Other 66 p/p/ha		IND 35,000 L/ha/day		MOE Chart 17000 L/ha/day		where P = population in thousands																			

velocity of approximately 1.4m/s (within MOE recommended forcemain velocities of 0.8 to 2.5m/s).

According to the *West Urban Community – Wastewater Collection System Master Servicing Plan* by RV Anderson Associates Ltd., dated July 2012 monitored peak flows entering the Stittsville Pump Station were 39L/s in 2010. With a capacity of 108L/s, the remaining capacity is 69L/s. Based on the aforementioned, the Liard St. Pump Station can handle the majority of the development. It is recommended that the flows at the Liard St. Pump Station continue to be monitored until extension of the Fernbank Trunk is completed (see 6.1.5 for details regarding the Future Fernbank Trunk).

6.1.4 Friendly Crescent Pump Station

The Friendly Crescent Pump Station is a low lift station, which services the properties along Friendly Crescent. The flow is pumped west to the 250mm dia. sewer along Hartsmere Drive and has an overflow that is directed to a storm outlet east of Friendly Crescent.

Novatech Engineering produced the “Design Services and Stormwater Report” in May 2000 with a detailed design of the Friendly Crescent Pump Station. The station was designed to serve 70 dwellings that discharge to the Friendly Crescent Pump Station with a peak flow of 5.77 L/s using twin Flygt effluent pumps CP3085.182 that push 6.0 L/sec at 7.15 meters total dynamic head through a 100mm diameter, 230m long forcemain.

It is proposed that the sanitary sewer-shed of Friendly Crescent Pump Station be accounted for in the servicing alternatives, in order to provide a higher level of service, by providing a gravity outlet to avoid the costs of maintaining and operating the existing pump station.

6.1.5 Future Fernbank Trunk

The Future Fernbank Trunk will be built along the Hydro One easement to accommodate the future development of the Fernbank Community Design Plans as referenced in the Master Servicing Study for the Fernbank lands. Once constructed, the Liard Street Pump Station will be decommissioned, and all flows from the Liard Street Pump Station sewer-shed and the Area 6 lands will be directed to the Fernbank Trunk through a gravity sewer. The Fernbank Trunk will convey flows to the Hazeldean Pump Station. The decommissioning work will be undertaken by the City, based on the time frame provided in Infrastructure Master Plan.

The Fernbank Trunk was designed for a peak flow of 528L/s and has a capacity of 670L/s which leaves an excess capacity of 142L/s. As per section 6.1.3 of this report, the Liard Street Pump Station had a monitored flow of 39L/s in 2010, and proposed Area 6 peak design flows is 85L/s which summates to 124L/s. Based on these flows, there is adequate capacity in the Fernbank Trunk.

Based on coordination with the Landowners within the Fernbank CDP lands, the sewer depth and size will be accounted for at the proposed subdivisions within the Fernbank Lands CDP to provide the required capacity in order to eventually decommission the Liard Street Pump station and accumulate the Area 6 flows. The cost for over-sizing and over –depth of the sewers is discussed in Section 9.2.

Table ES 3: WUC summary of flow generation scenarios

PUMPING STATION OR TRUNK SEWER	FIRM CAPACITY	EXISTING CAPACITY	CURRENT SEWER CONFIGURATION						
			FLOW ⁽¹⁾	Scenario 1		Scenario 2		Scenario 3	
				(L/s)	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)
(Year)			2010	2031	2060	2031	2060	2031	2060
	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)	(L/s)
Richmond Pump Station	360		151	340	340	314	314	407	407
Stittsville PS	108		39	106	506	77	300	91	353
Hazeldean Pump Station	1225		832	1537	1937	1373	1596	1741	2003
Kanata West Pump Station ⁽²⁾	765		152	593	689	462	555	561	678
Signature Ridge Pump Station ⁽³⁾	360		54	309	423	218	302	256	351
March Pump Station	490		326	771	941	668	814	820	1008
Acres Road Pump Station	4600		2119	4186	4966	3774	4320	4437	5099
Glen Cairn Trunk		2815 to 2988	1139	2512	3008	2192	2508	2758	3137
Stittsville Trunk		519 to 972	358	485	885	444	679	572	732
Main Street Sewer		307 to 739	138	330	444	237	321	342	399
Penfield Sewer		398 to 734	170	360	474	267	351	342	437
March Ridge Trunk (Above March Forcemain)		1223	245	434	548	339	423	428	523
March Ridge Trunk (Below March Forcemain)		1016	571	1205	1489	1007	1237	1248	1531
Watts Creek Siphon		1014	571	1205	1489	1007	1237	1248	1531
Tri-Township Collector		1595 to 1803	1705	3717	4497	3199	3745	4006	4668
March Wood Trunk		1100	230	574	705	502	616	608	752
East March Trunk		550	96	172	211	141	173	187	231
North Kanata Trunk - Phase I		4047 to 4640	1705	3717	4497	3199	3745	4006	4668
Nepean Collector		190	190	197	197	193	193	234	234
Watt's Creek Trunk		5418 to 6640	1891	3914	4694	3392	3938	4240	4902

■ The coloured cells in the table identify the component of the current sewer system that is under capacity by the time of the projected growth in 2031 or 2060.

⁽¹⁾ – flow results based on the dynamic model calculation;

APPENDIX C

WATER SUPPLY

**STITTSVILLE SOUTH SUBDIVISION
CITY OF OTTAWA**

DETAILED SERVICING & STORMWATER MANAGEMENT REPORT

Prepared For:

ROSS BRADLEY, CINQUE TERRE HOLDINGS INC. & STITTSVILLE SOUTH INC.

Prepared By:

NOVATECH

Suite 200, 240 Michael Cowpland Drive
Ottawa, Ontario
K2M 1P6

September 21, 2015
Revised July 18, 2016

Novatech File: 113004
Ref: R-2015-072

6.0 WATER SUPPLY SYSTEM

6.1 Background Information

The proposed development is located within the service area of Pressure Zone 3W of the City of Ottawa water distribution system. This zone is fed by the Glen Cairn and Campeau Drive Pump Stations. Balancing storage during peak and fire flow conditions is accomplished with use of the Stittsville Elevated Tank.

The existing water distribution system adjacent to the study area includes;

- 400mm diameter watermain on Fernbank Road
- 300mm diameter watermain on Arrowwood Drive
- 200mm diameter watermain on Hartsmere Drive
- 250mm diameter watermain on West Ridge Drive

The existing ground elevations within Area 6 are between 105m and 124m above sea level.

Novatech has retained Stantec Consulting Ltd.(Stantec) to undertake a hydraulic analysis using the City's most up to date model which had recently been updated for the 2013 Water Master Plan update. The analysis takes into account the future Fernbank CDP Lands. The Stantec findings and recommendations; '*Stittsville Area 6 - Potable Water Hydraulic Assessment Phase 1 & 2*' dated September 2nd, 2015 can be found in **Appendix D**.

6.2 Previous Recommendations

As part of the *Area 6 MSR*, it was determined that the preferred water servicing alternative consists of a 250mm watermain spine, and 200mm diameter watermain feeding adjacent local roads. It was also recommended that a 300mm diameter watermain be extended from Arrowwood Drive into the proposed site as 200mm watermain for backbone continuity, and the 400mm diameter watermain on Fernbank Road be extended towards the East in order to meet the proposed 250mm diameter spine. Refer to **Figure 6.1**.

6.3 Overall Water Demand & Criteria

The water demands for Stittsville South would be estimated using the City of Ottawa's Water Distribution Design Guidelines.

6.3.1 Water Demand

The domestic demand design criteria used to determine the size of the watermains required to service the Stittsville South area are as follows:

Domestic Demand

Average Residential Domestic Flow per capita	350 L/cap/day
Capita per dwelling	3.4 persons per single
	2.7 persons per townhouse
	2.3 persons per stacked townhouse
	2.1 persons per apartment
Maximum Day Demand	2.5 x Average Day Demand
Peak Hour Demand	2.2 x Maximum Day Demand

Commercial and Parks Demand

Commercial Capita	50,000 L/ha/day
Maximum Day Demand	1.5 x Average Day Demand
Peak Hour Demand	1.8 x Maximum Day Demand
Park Demand	1000 L/park/day

6.3.2 Fire Flow Demand

The City of Ottawa requires proposed watermain networks meet Fire Underwriters Survey fire flow requirements. However, Technical Bulletin ISDTP-2014-02 specifies that the fire flow requirement can be capped at 10,000 L/min for the following;

- Single detached dwellings, provided that there is a minimum rear yard separation of 10m between adjacent units.
- Town and row homes, provided that firewalls with a minimum of two hour fire-resistance rating that comply with OBC Div. B, Subsection 3.1.10 are used to separate home blocks into fire areas that comprise no more than the lesser of seven units, and 600m² of building area. Furthermore, there must be a minimum rear yard separation of 10m.

Based on the above, the watermain analysis has assessed the ability of the proposed network to attain a fire flow of 10,000 L/min at all locations. Fire Underwriters Survey fire flow calculations have also been included for reference in **Appendix D**.

6.3.3 Design Criteria

The design criteria used to determine the size of the watermains required to service the Stittsville South area are based on a conservative approach that considers three possible scenarios, as follows:

System Pressures

Maximum Allowable Pressure	551.6kPa (80psi)
Minimum Allowable Pressure (excluding fire flow conditions)	275.8kPa (40psi)
Minimum Allowable Pressure (including fire flow conditions)	137.9Kpa (20psi)

6.4 Watermain Analysis

Novatech has retained Stantec Consulting Ltd. to conduct a hydraulic analysis of the proposed development potential, and its effects on the City's water infrastructure. The hydraulic network model and memo 'Stittsville Area 6-Phase 1 & 2 - Potable Water Hydraulic Assessment' dated September 2nd, 2015 is included in **Appendix D**. The hydraulic network simulated average day, peak hour and maximum day plus fire flow conditions.

Stantec used the City's most up to date model that was recently updated for the 2013 Water Master Plan. Both current conditions and future conditions (anticipated 2031 conditions from the 2031 Water Master Plan model) were analyzed.

It is important to note that in the area of the proposed development, head losses under peak demands could reduce minimum pressures to below guideline requirements at higher

elevations. Future planned connections as per the Water Master Plan, within the Fernbank lands will mitigate this issue resulting in increased minimum pressures.

6.5 Discussion

6.5.1 Low Pressures

Under peak hour demands, ground elevations greater than 124m are susceptible to minimum pressures marginally below the required 40psi under 2013 existing conditions. In future 2013 conditions, minimum pressures everywhere within the proposed development are not expected to drop below required pressures.

Within the vicinity of the cul-de-sac at the end of Street Five, ground elevations are greater than 124m. In order to mitigate marginally low expected pressures, it is proposed that 25mm services be installed for Lots 23, 24, and 25 to alleviate low pressure concerns. The specifications and details of these mitigations will be provided in the detail design drawings of the Camplina Way, 113004-GP1.

It is also expected that buildings within Block 349 will experience marginally low pressures due to the ground elevations in this area. Within Block 349, jet pumps will be required where buildings are greater than two stories tall. The jet pumps will be owned and maintained by the condominium corporation. Such mitigation measures, including the jet pumps, will be finalized within servicing reports during detailed design in support of the site plan application.

Similarly, at Block 353 contains the potential for a 6 story condominium building that will likely require pressure boosting measures. Such measures include but are not limited to jet pumps within the mechanical room of the apartment block. These measures will be owned and maintained by the condominium corporation and will be finalized within servicing reports during detailed design in support of the site plan application.

Refer to the Legal Plan of Subdivision for the location of the aforementioned blocks.

6.5.2 High Pressures

Under average day demands (also known as basic day demands), ground elevations less than 106m will experience pressures greater than the required limit of 80psi. As Phase 1 and 2 of the Stittsville South subdivision do not have any finished grade elevations around residential units or street below elevation 106.00, high pressures are not a concern.

6.5.3 Fire Flow

As per Appendix B-5 through B-8 of the attached Stantec Potable Water Hydraulic Assessment, a 10,000L/min fire flow is attained in general throughout the proposed development as per ISDB-TB2014-01. However there are a few localized exceptions, where the criteria was not met as discussed below.

At node A95 (at the dead end of Campolina Way) the available fire flow is 8000L/min. The FUS long calculation for Lot 25 (worst case scenario) was calculated to be 8,000L/min; therefore, the fire demand is met. a second 250mm watermain loop (along Falabella, Campolina, Lipizzaner) was introduced to retain the minimum required fire flows of 8,000L/min.

As per ISD-TB 2014-01, at node 81, the residential configuration does not allow the 10,000 L/min fire demand to be utilized and requires the FUS long method to be utilized. As such, the fire demand at this location is 13,000 L/min. The available fire flow at residual pressure of 20 psi, is 18,000L/min; therefore, the fire demand is met.

6.6 Recommendations & Proposed Water Infrastructure

Based on the findings of 'Stittsville Area 6 - Potable Water Hydraulic Assessment' there is sufficient capacity to provide both the required domestic and emergency fire flows to the service area. In order to accomplish this, it is proposed that the 250mm diameter watermain existing west of the development would be extended through Area 6 and reconnected at Fernbank Road. This 250mm watermain would act as the spine, from which 200mm diameter watermain will feed adjacent roads. It is also recommended that an existing 300mm diameter watermain be extended as a 200mm watermain from Arrowwood Drive into the proposed site for backbone continuity, and the 400mm diameter watermain on Fernbank Road be extended towards the East in order to meet the proposed 250mm diameter spine. Elevations greater than 124m will require additional measures to increase peak hour pressures. Elevations less than 106m will require pressure reduction measures. Refer to **Figure 6.1** for sizing.

It is likely that the eastern portion of Parade (Lots 286 – 295) will be developed in advance of the Davidson Lands; hence the watermain within this portion will be deemed a dead-end. Is it anticipated that the Davidson Lands will advance within the next two years. As there are less than 50 units temporarily connected to this portion of the main, the City's guidelines are met.

M:\2013\113004\CAD\Design\Figures\Design\Figures.dwg, FIG 6.1, Jul 14, 2016 - 11:24am, bsweet



NOTE: DAVIDSON LANDS LAYOUT IS SUBJECT TO CHANGE (BY OTHERS). REFER TO CITY OF OTTAWA FILE NO. D07-16-15-0008.

LAYOUT TO BE CONFIRMED BY DAVIDSON PARCEL DRAFT PLAN PROCESS (BY OTHERS)

SCALE 1:7000



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STITTSVILLE SOUTH AREA 6

WATERMAIN CONCEPT PLAN

DATE	JULY 2016	JOB	113004	FIGURE	FIGURE 6.1
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APPENDIX D

Hydraulic Analysis – Stantec (Retained by Novatech)

**Stittsville Area 6 - Potable
Water Hydraulic Assessment
of Phase 1 & 2**



Prepared for:
Novatech Engineering
Consultants Limited

Prepared by:
Stantec Consulting Ltd.

September 2, 2015

Sign-off Sheet

This document entitled Stittsville Area 6 - Potable Water Hydraulic Assessment of Phase 1 & 2 was prepared by Stantec Consulting Ltd. ("Stantec") for the account of Novatech Engineering Consultants Ltd. (the "Client"). Any reliance on this document by any third party is strictly prohibited. The material in it reflects Stantec's professional judgment in light of the scope, schedule and other limitations stated in the document and in the contract between Stantec and the Client. The opinions in the document are based on conditions and information existing at the time the document was published and do not take into account any subsequent changes. In preparing the document, Stantec did not verify information supplied to it by others. Any use which a third party makes of this document is the responsibility of such third party. Such third party agrees that Stantec shall not be responsible for costs or damages of any kind, if any, suffered by it or any other third party as a result of decisions made or actions taken based on this document.



Prepared by _____
(Signature)

Val Hoang, M.A.Sc., Engineering Intern



Reviewed by _____
(Signature)

Kevin Alemany, M.A.Sc., P.Eng.

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Executive Summary

Stantec Consulting Ltd. (Stantec) has carried out a detailed potable water hydraulic analysis for Phases 1 and 2 of the proposed Area 6 service area located in Stittsville on behalf of Novatech Engineering Consultants Ltd. The proposed Area 6 development is located between Stittsville Main Street and Shea Road along Fernbank Road and is adjacent to the boundaries of Pressure Zone 3W of the City of Ottawa water distribution system.

A hydraulic assessment was performed using the City's most up to date model (with permission) for existing conditions to simulate Phase 1 and 2. The spine of the network is proposed to be 250mm diameter piping (which connects to existing watermains) with 200mm diameter piping making up the remainder of the internal network.

The proposed watermain to service the mixed use development has sufficient capacity to provide the required domestic demands while maintaining the City's objective pressure in the development. However, additional consideration should be taken for buildings with more than two storeys in height as they are subject to experiencing low pressures (i.e. below 40 psi) on the higher floors during peak demands. Proposed building heights, ground elevations and minimum pressure constraints need to be considered accordingly.

A fire flow assessment under maximum day demand conditions was carried out and it was determined that fire flows greater than 10,000 L/min can be achieved while maintaining a residual pressure of 20 psi throughout the development except for one dead-end location. According to the latest site plans, this dead-end location is anticipated to service units that require 8,000 L/min of fire flow per the FUS calculation, which is deemed achievable according to model results presented herein.

STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

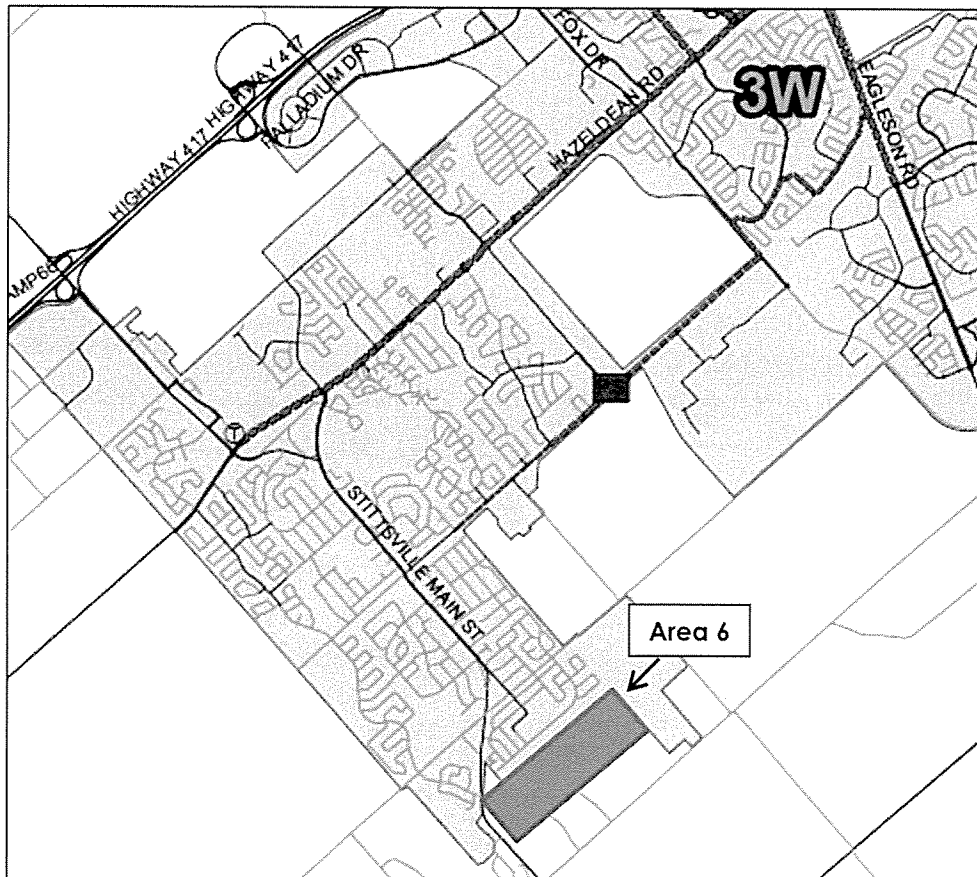
1.0 BACKGROUND

1.1 AREA OF DEVELOPMENT

Stantec Consulting Ltd. (Stantec) has undertaken a hydraulic assessment of the potable water servicing area for the proposed Area 6 Stittsville development on behalf of Novatech Engineering Consultants Ltd. This analysis specifically reviews conditions of Phase 1 and 2, as they are currently in the detailed design stage. Area 6 is a mixed-use development that includes single homes, town houses, apartment buildings and a small commercial area.

The proposed development site is located between Stittsville Main Street and Shea Road along Fernbank Road (Figure 1-1). It is located within the service area of Pressure Zone 3W of the City of Ottawa water distribution system. Zone 3W is fed by the Glen Cairn and Campeau Drive Pump Stations with the Stittsville Elevated Tank providing balancing storage for peak flows as well as it provides storage to meet emergency and fire flow conditions.

Figure 1-1: Location of Area 6



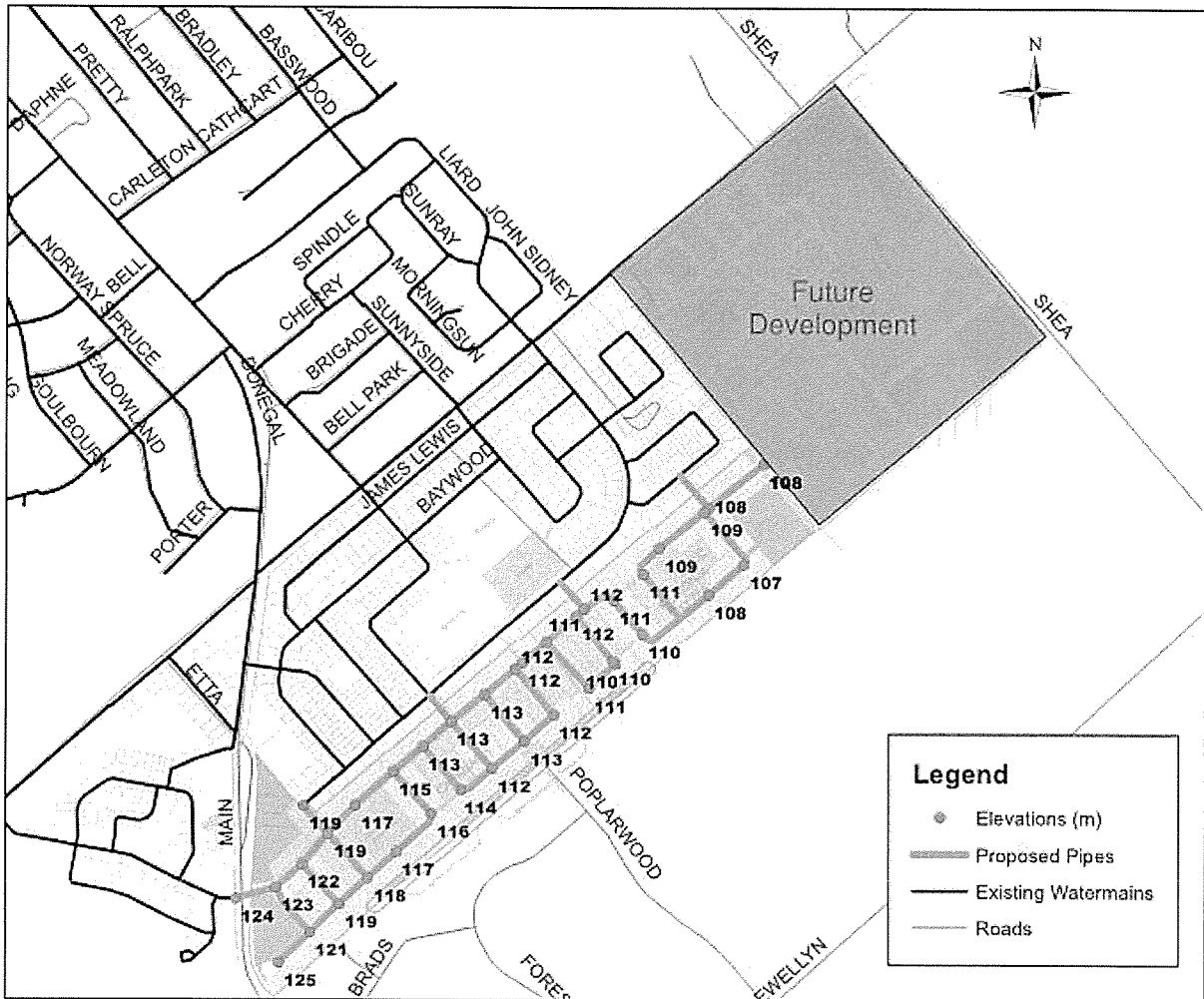
STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

1.2 GROUND ELEVATIONS

The existing ground elevations of the proposed Area 6 development range from approximately 107m and 125m. The elevations shown on **Figure 1-2** were interpolated from an elevation topography file and assigned to the nodes in the hydraulic model.

Figure 1-2: Ground Elevations (m) in Area of Proposed Development



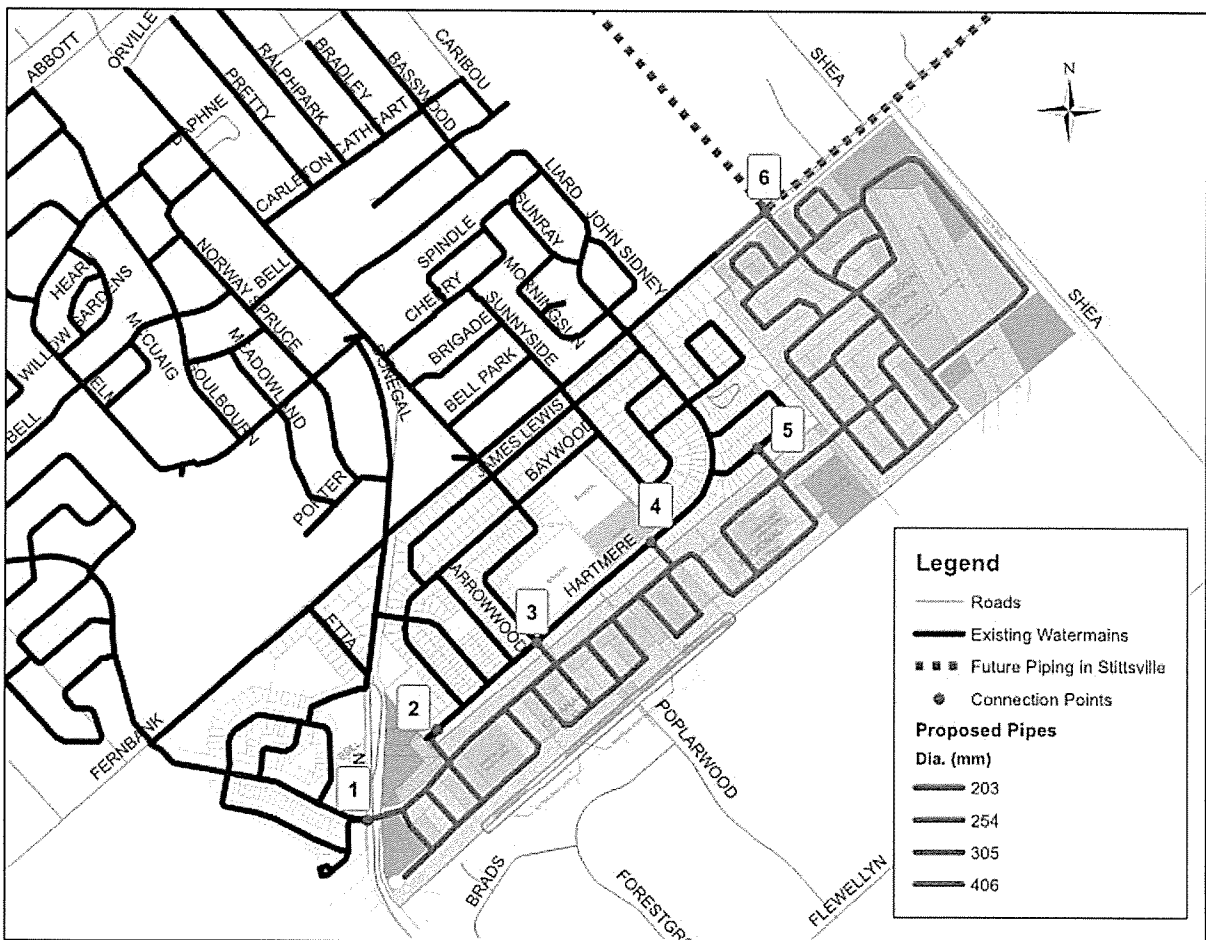
STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
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1.3 PREVIOUS STUDIES

In 2014, Stantec performed a Zone Level hydraulic analysis for Stittsville Area 6 where the criteria described in the 2013 Water Master Plan (WMP) was used to estimate water demands for the entire development. The resulting proposed watermain network is shown in **Figure 1-3** along with the connection points of Area 6 to the existing water distribution system. Pipes within the proposed network are made up of 305mm, 250mm and 203mm in diameter.

Figure 1-3: Connection Points of Area 6 to Existing Watermains from Stantec 2014 Report



Hydraulic modelling showed that the proposed pipe sizing and alignment was capable of providing domestic demand and the City's Objective fire flow of 10,000 L/min while maintaining pressures in accordance to the City Guidelines except for one location. This location is the dead-end located in the southwest (cul-de-sac) of the development which was capable of achieving 7,000 L/min of flow. Additionally, since this location has a ground elevation greater than 124.5m, it resulted in minimum pressures slightly below the City's objective of 40 psi during peak hour demands. It was recommended that oversized services and plumbing be considered



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

for this location to achieve minimum pressures of 40 psi. Constraints of multi-storey buildings located in areas of high elevations should also be considered accordingly to avoid low pressure on the higher storeys.

During analysis of Phase 3 which was modeled under future 2031 conditions, it was noted that the typical operating pressures are anticipated to exceed the objective limit of 80 psi in the lands with lower elevations. Areas with ground elevations less than 106m are expected to experience pressures greater than 80 psi and require pressure reducing measures to be in accordance with the Ontario Plumbing.

1.4 PHASING & PROPOSED PIPING

Area 6 is currently proposed to be developed in 3 phases (**Figure 1-4**). Phase 1 and 2 are currently in the detailed design stage while Phase 3 is not expected to be developed in the short term and not included in this latest servicing analysis.

Phase 1 - the Cavanagh lands includes

- a connection to the existing 250mm diameter watermain on West Ridge Dr. (point 1);
- a connection the existing 203mm diameter watermain on Hartsmere Dr. (point 2).

Phase 2 - Regional lands and Bell Lands west of Cavanagh Lands includes

- an extension of 300mm diameter watermain along Arrowwood Dr. (point 3);
- a connection to the existing 203mm diameter watermain on Hartsmere Dr. (point 4); and
- a connection to the existing 203mm diameter watermain on Friendly Cres. (point 5).

Phase 3 - Davidson Lands (to be developed in future phases)

- construction of a small section of 400mm diameter pipe to connect point 6 to the existing watermain on Fernbank; this extension would represent a portion of the extended future 400mm diameter watermain along Fernbank Road towards Shea Road.



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

Figure 1-4: Phasing Plan

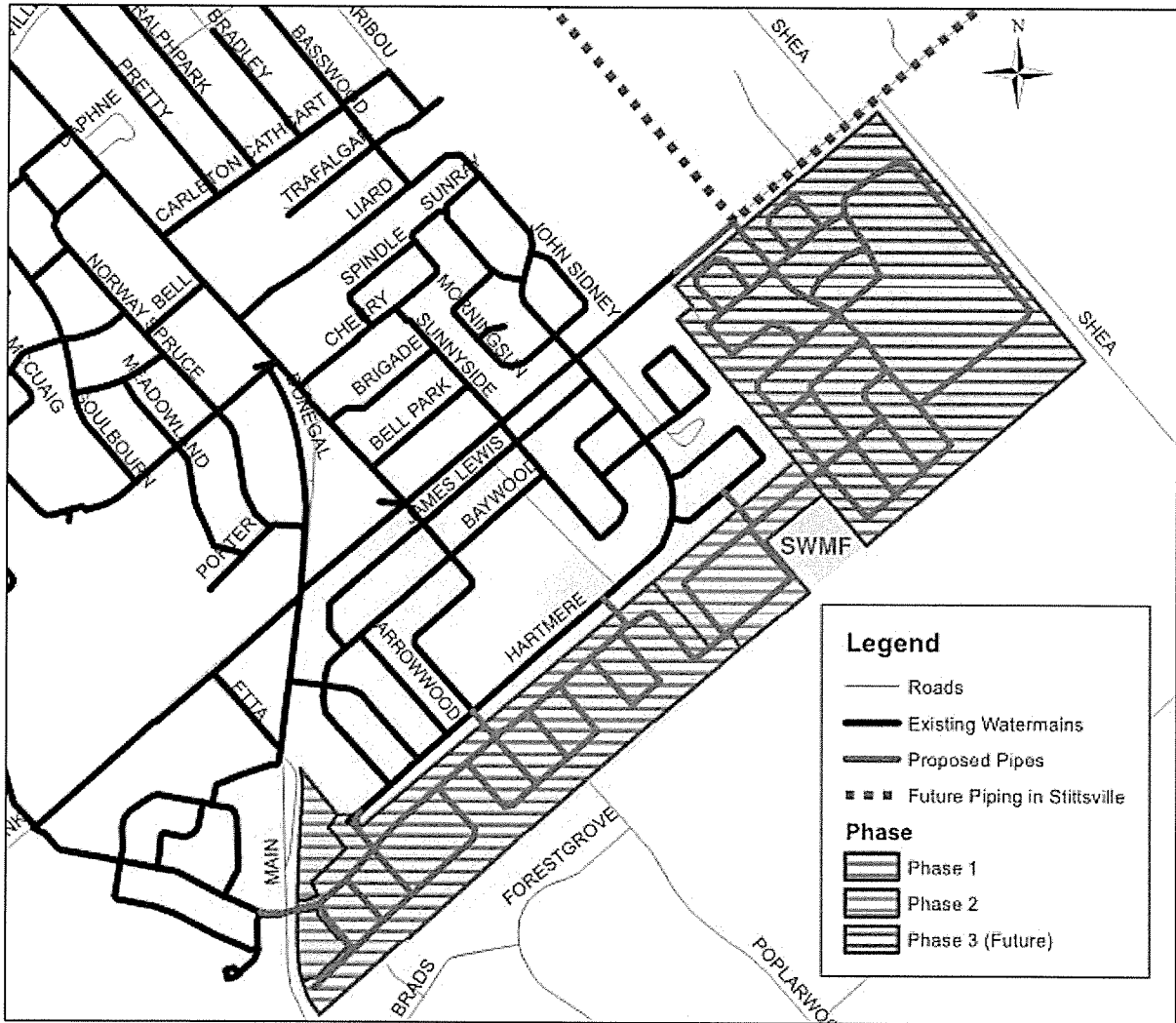


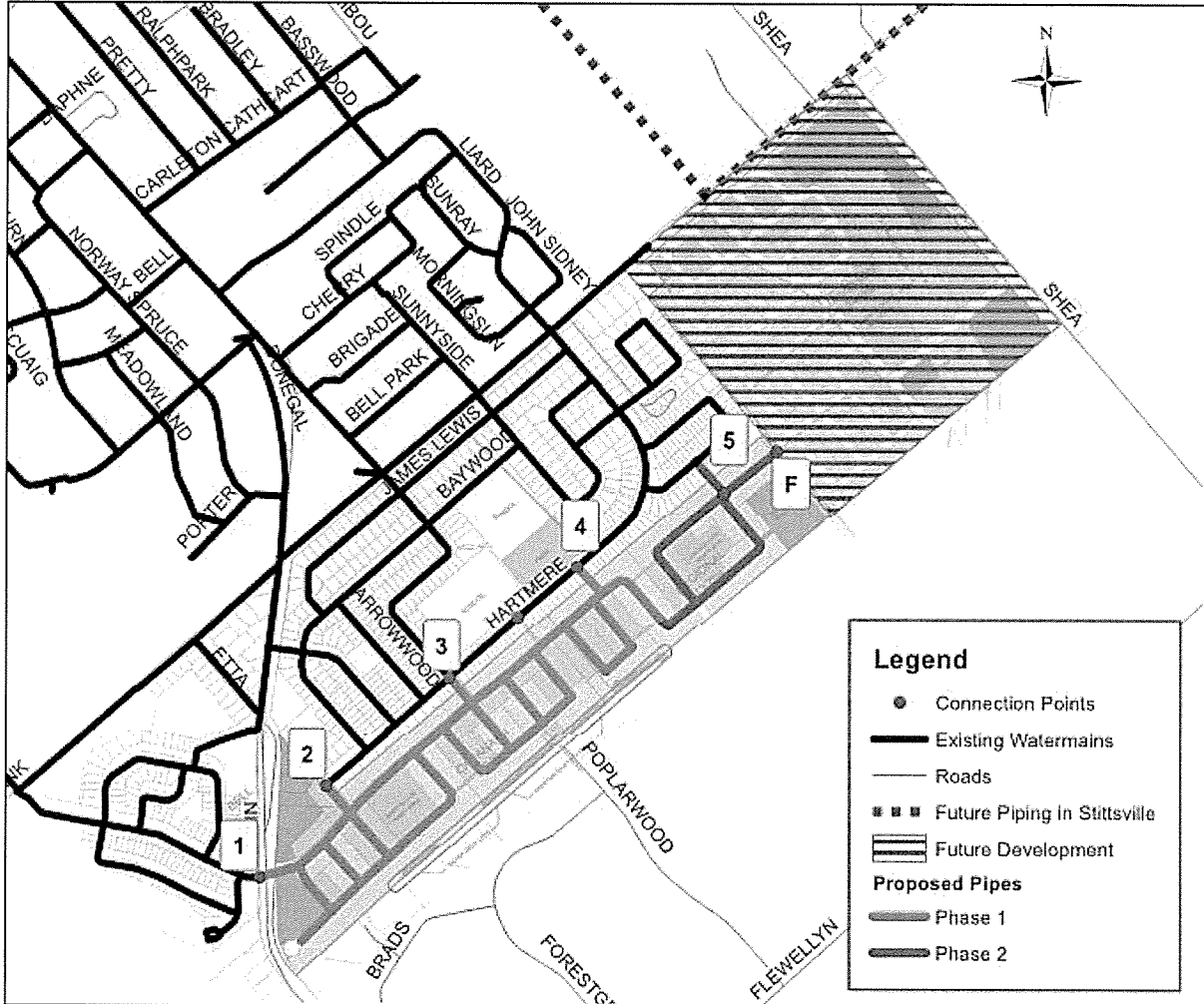
Figure 1-5 shows the connection points of the development to the existing watermain network and the connection from Phase 2 to the future development (Phase 3) on the east side of Area 6 (denoted as connection "F"). It should be noted that the watermain from connection 3 into the development (along Arrowwood Drive) as shown in Figure 1-6 was previously proposed to be a 305mm diameter pipe (refer to Figure 1-3) but has been revised to 250mm and 203mm pipes to avoid oversized piping. Additionally, to increase the and minimum pressures during peak hour and fire flow at the cul-de-sac location, the surrounding pipes are recommended to be upsized from 203mm to 250mm diameter watermains.



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

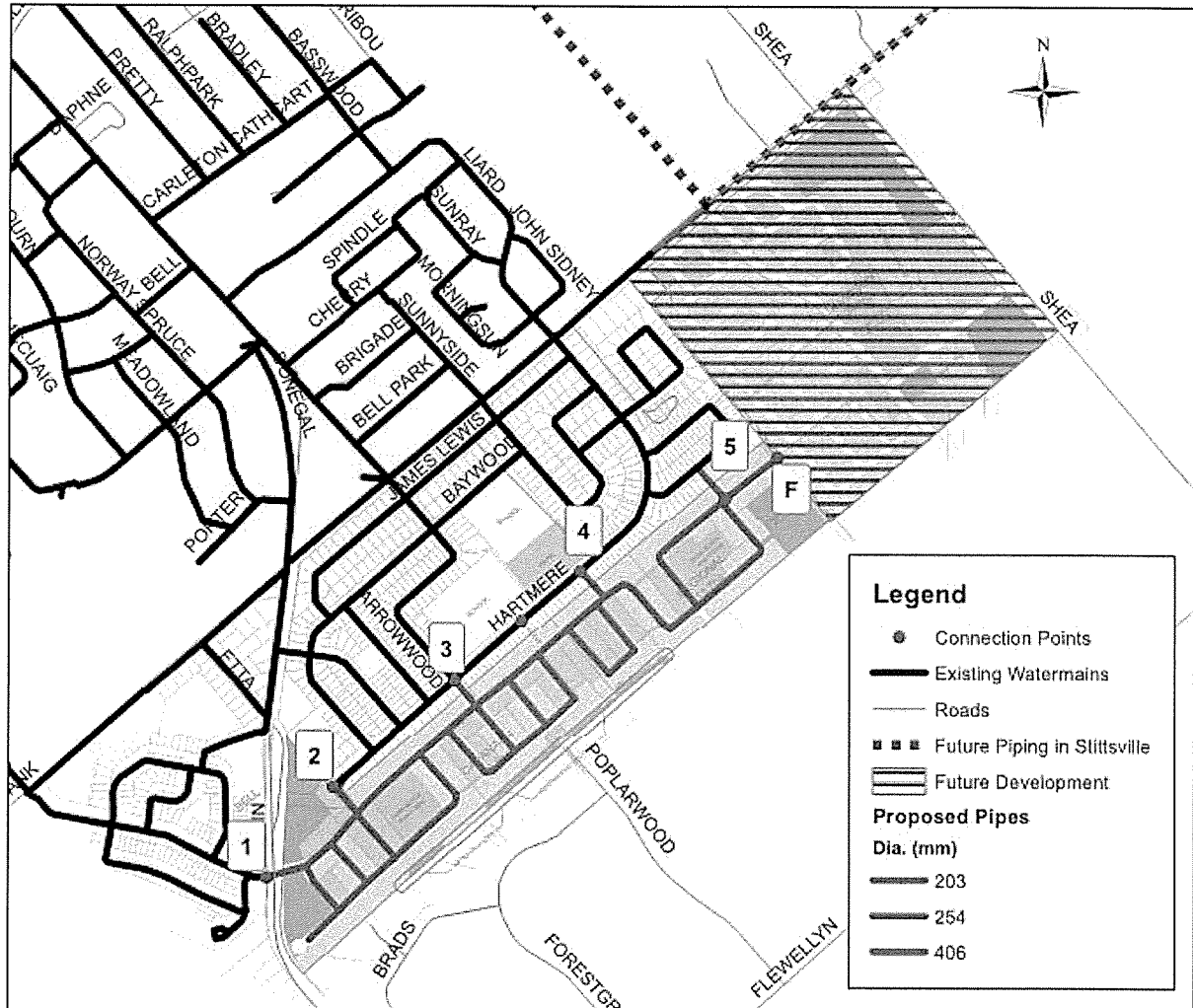
Figure 1-5: Connection Points to Existing System and Future Development



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Background
September 2, 2015

Figure 1-6: Proposed Piping Sizing and Alignment



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Serviceability
September 2, 2015

2.0 SERVICEABILITY

2.1 ALLOWABLE PRESSURES

The City of Ottawa Water Distribution Design Guidelines state that the design objective for system pressures under normal demand conditions (i.e. average day, maximum day and peak hour) is in the range of 40 to 80 psi at the ground elevation in the streets (i.e. at hydrant level). The maximum pressure at any point in the distribution system is 100 psi; however, as per the Ontario Building/Plumbing Code, pressure relief measures are required for services when pressures greater than 80 psi are anticipated. Under emergency fire flow conditions, the minimum pressure in the distribution system is allowed to drop to 20 psi.

Multi-storey residential buildings require an additional 5 psi for every additional storey over two storeys to account for the change in elevation head and some additional headloss. For example, the minimum pressure required for a two-storey building is 40 psi whereas a three-storey building requires at least 45 psi and a four-storey building requires at least 50 psi. This is to account for the difference in elevation and additional pipe headloss.

2.2 FIRE FLOWS

The City of Ottawa requires new developments to demonstrate that the proposed watermain network can achieve the Fire Underwriters Survey (FUS) fire flow objective (using the long form calculation). The City's Technical Bulletin ISDTB-2014-02 specifies the type of development and condition that allow fire flow requirements to be capped at 10,000L/min.

Novatech has confirmed that the maximum fire flow requirement that would be required based on the planned development is 10,000L/min. FUS calculations are presented in **Appendix A**. This report assesses the ability of the network to attain a fire flow of 10,000L/min throughout the network.

STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Water Demands
September 2, 2015

3.0 WATER DEMANDS

The City of Ottawa's Water Design Guidelines were used for estimate the water demands of Phase 1 and 2 as they are in the detailed design stage of development. The average day (AVDY) demands were estimated using a residential consumption rate of 350 L/cap/d and population densities based on various unit types. **Table 3-1** shows the unit count and estimated population. For parks, a water consumption rate of 1,000L/d was applied. **Table 3-2** shows the total park area for Phase 1 and 2 and its corresponding water demand.

Maximum day (MXDY) demands were estimated by multiplying AVDY demands by a factor of 2.5 and peak hourly (PKHR) demands were estimated by multiplying MXDY demands by a factor of 2.2. **Table 3-3** shows the estimated water demands for each phase where the latest plans for Phase 1 and 2 calls for a total **650** units and an estimated population of **1,903**.

Table 3-1: Estimated Residential Population based on Unit Types

Phase	Unit Type	Persons/Unit	Units	Population
1	Single Family	3.4	269	914
	Town Houses	2.7	126	341
	Phase 1 Total		395	1,256
2	Single Family	3.4	69	235
	Town Houses	2.7	34	92
	Apartments	2.1	152	320
	Phase 2 Total		255	647
			650	1,903

Table 3-2: Non-Residential Areas

Phase	Non-Residential	Area (ha)	Demand (L/s)
1	Park	1.33	0.77
2		0.82	0.47

Table 3-3: Estimated Water Demand

Phase	Population	BSDY (L/s)	MXDY (L/s)	PKHR (L/s)
1	1,256	5.08	12.71	27.96
2	1,903	7.70	19.25	42.34

STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Hydraulic Modelling Results
September 2, 2015

4.0 HYDRAULIC MODELLING RESULTS

4.1 HYDRAULIC MODEL SET-UP

With the permission of the City, Stantec performed the hydraulic analysis using the City's 2013 Water Master Plan (WMP) model. Stantec assessed the anticipated pressures in the Area 6 development and reviewed potential upgrades/upsizing of existing watermains (if any) in order to meet minimum servicing requirements.

The software package used to carry out the analysis was H₂OMAP Water by Innovyze. The model was tested under three different domestic demand conditions: basic day (BSDY), peak hour (PKHR) and one emergency condition: maximum day plus fire (MXDY+FF). For the analysis herein, Stantec adjusted the previous model that was used for the 2014 hydraulic analysis to correspond to the updated changes of Phase 1 and 2.

New watermains were added to the hydraulic model to simulate the proposed distribution system. Hazen-Williams coefficients ("C-Factors") were applied to the new watermain in accordance with the City of Ottawa's Water Distribution Design Guidelines (**Table 4-1**):

Table 4-1: C-Factors Used for Applied Watermain Based on Pipe Diameter

Pipe Diameter (mm)	C-Factor
150	100
200 to 300	110
350 to 600	120
> 600	130

4.2 BASIC DAY AND PEAK HOUR DEMANDS

Steady-state modelling under 2013 (existing) conditions was used to model basic day and peak hour scenarios. **Table 4-2** shows the pressure observed during hydraulic modelling under BSDY and PKHR demands. It can be seen that maximum pressures do not exceed the City's objective of 80 psi in Phase 1 and 2.

During the previous 2014 study, one location within Area 6 with a ground elevation greater than 124m (cul-de-sac) was susceptible to minimum pressure marginally below 40 psi under peak hour. As such for this analysis herein, the pipes surrounding this area were upsized from 200mm to 250mm diameter watermains to increase the minimum pressures to the City's objective. Hydraulic modelling results show that in doing so, the minimum pressure at the cul-de-sac is at 40 psi and satisfies the City's guidelines. It is recommended that pressure testing be performed to confirm that pressures do not drop below 40 psi in this location.



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Hydraulic Modelling Results
September 2, 2015

Table 4-2: Residual Pressure during BSDY and PKHR Demands

Phase	BSDY	PKHR
	Max. Pressure (psi)	Min. Pressure (psi)
1	72	40
2	76	40

Multi-storey buildings require an additional 5 psi for every additional storey over two storeys to account for the change in elevation head and additional headloss. Based on a resulting minimum hydraulic gradeline of 153m under peak demand conditions in Area 6, the following are the "cut-off" elevations for various multiple storey buildings, above which, pressures would fall below the minimum pressure guideline objective:

Two storeys: ground elev. greater than 124.5m results in pressures less than 40 psi.

Three storeys: ground elev. greater than 121.0m results in equivalent pressures less than 40 psi.

Four storeys: ground elev. greater than 117.5m results in equivalent pressures less than 40 psi.

4.3 MAXIMUM DAY + FIRE FLOW

The City of Ottawa's design guidelines for water distribution systems require a minimum pressure of 20 psi to be maintained at all points in the distribution system under a condition of maximum day and fire flow demand. A hydraulic analysis was accomplished using a steady-state maximum day demand scenario along with the automated fire flow simulation feature of the software.

During 2013 conditions for Phase 1 and 2, the proposed network was able to supply fire flows greater than 10,000 L/min while maintaining a residual pressure of 20 psi at all locations in Area 6 except one location in the southwest of the development. This location was modelled as a dead-end and the available fire flow was approximately 8,000 L/min at a residual pressure of 20 psi. However, the latest site plan shows this area is anticipated to service single family homes and as per the FUS fire flow calculated for this unit type, the required fire flow is 8,000 L/min (**Appendix A**). See **Appendix B** for available fire flows as each location.

STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Conclusion
September 2, 2015

5.0 CONCLUSION

The proposed mixed residential development is located in an area of the City's water distribution system that has sufficient capacity to provide both the required domestic and emergency fire flows to the majority of the service area using the proposed piping alignment and sizing proposed.

During existing conditions, Phases 1 and 2 are expected to operate in objective range of 40 - 80 psi under BSDY and PKHR demands. It is recommended, however, that the dead-end location (cul-de-sac) located in the southwest portion of the development be checked for pressures to confirm minimum pressures do not drop below 40 psi.

A fire flow analysis was performed and it was determined that a fire flow greater than 10,000 L/min is achievable at all locations except for one dead-end located in the southwest portion of the development. Site plans show that location is anticipated to service single family homes and will require an FUS fire flow of 8,000L/min which is achievable.



STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Appendix A FUS Fire Flow Calculations
September 2, 2015

**Appendix A FUS FIRE FLOW
CALCULATIONS**

Fire Flow Calculations - Towns (w/o Party Walls) (6 Unit Row)

As per Fire Underwriter's Survey Guidelines

PROJECT: Area 6 Lands
JOB#: 113004

DATE: Aug 2015

C Coefficient related to type of construction		[yes/no]	
♦ Wood frame	y		1.5
♦ Ordinary construction			1
♦ Non-combustible construction			0.8
♦ Fire resistive construction (< 2 hrs)			0.7
♦ Fire resistive construction (> 2 hrs)			0.6
♦ Interpolation (Using FUS Tables)			
A Area of structure considered (m²) *	1,200	<==>	12,917 ft²
<i>(All floors excluding Basement, under 2-Storeys)</i>			
F Required fire flow (L/min)			<u>11,432 L/min</u>
$F = 220 C (A)^{0.5}$			
Occupancy hazard reduction of surcharge		[yes/no]	
♦ Non-combustible	y		-25%
♦ Limited combustible			-15%
♦ Combustible			0%
♦ Free burning			15%
♦ Rapid burning			25%
			<u>8,574 L/min</u> (1)
Sprinkler Reduction			
♦ Non-combustible - Fire Resistive (3)	no		50% <u>0 L/min</u> (2)
Exposure surcharge (cumulative (%), 2 sides)		[yes/no]	
0 - 3 m	yes		25% 2 side 50%
3.1 - 10 m			20%
10.1 - 20 m	yes		15% 1 side 15%
20.1 - 30 m	yes		10% 1 side 10%
30.1- 45 m			5%
Cumulative Total			75%
			6,430 L/min
Fire Wall Separation		N/A	
♦ Number of Party Walls * 1000 L/min			<u>6,430 L/min</u> (3)
<i>(As per City of Ottawa Standard)</i>			
REQUIRED FIRE FLOW [(1) - (2) + (3)]			15,000 L/min
(2,000 L/min < Fire Flow < 45,000 L/min)			or 250 L/s
			or 3,303 IGPM
BY: Adam Lambros			
* Largest Block Size			

Fire Flow Calculations - Towns (w/o Party Walls) (Blocks 344&343)

As per Fire Underwriter's Survey Guidelines

PROJECT: Area 6 Lands
JOB#: 113004

DATE: Aug 2015

C Coefficient related to type of construction		[yes/no]	
♦ Wood frame		y	1.5
♦ Ordinary construction			1
♦ Non-combustible construction			0.8
♦ Fire resistive construction (< 2 hrs)			0.7
♦ Fire resistive construction (> 2 hrs)			0.6
♦ Interpolation (Using FUS Tables)			
A Area of structure considered (m²) *	1,000	<==>	10,764 ft²
<i>(All floors excluding Basement, under 2-Storeys)</i>			
F Required fire flow (L/min)			
F = 220 C (A) ^{0.5}			<u>10,436 L/min</u>
Occupancy hazard reduction of surcharge		[yes/no]	
♦ Non-combustible		y	-25%
♦ Limited combustible			-15%
♦ Combustible			0%
♦ Free burning			15%
♦ Rapid burning			25%
			<u>7,827 L/min</u> (1)
Sprinkler Reduction			
♦ Non-combustible - Fire Resistive (3)		no	50%
			<u>0 L/min</u> (2)
Exposure surcharge (cumulative (%), 2 sides)		[yes/no]	
0 - 3 m		yes	25% 1 side 25%
3.1 - 10 m		yes	20% 1 side 20%
10.1 - 20 m			15%
20.1 - 30 m		yes	10% 1 side 10%
30.1- 45 m		yes	5% 1 side 5%
			Cumulative Total 60%
			4,696 L/min
Fire Wall Separation			
♦ Number of Party Walls * 1000 L/min		N/A	
<i>(As per City of Ottawa Standard)</i>			<u>4,696 L/min</u> (3)
REQUIRED FIRE FLOW [(1) - (2) + (3)]			13,000 L/min
(2,000 L/min < Fire Flow < 45,000 L/min)			or 216.67 L/s
			or 2,862 IGPM
BY: Adam Lambros			
* Largest Block Size			

Fire Flow Calculations - Single Residential Unit (At Cul-De-Sac)

As per Fire Underwriter's Survey Guidelines

PROJECT: Area 6 Lands
JOB#: 113004

DATE: Aug 2015

C Coefficient related to type of construction		[yes/no]	
♦ Wood frame	y		1.5
♦ Ordinary construction			1
♦ Non-combustible construction			0.8
♦ Fire resistive construction (< 2 hrs)			0.7
♦ Fire resistive construction (> 2 hrs)			0.6
♦ Interpolation (Using FUS Tables)			
A Area of structure considered (m²) *		360	<==> 3,875 ft²
<i>(All floors excluding Basement, under 2-Storeys)</i>			
F Required fire flow (L/min)			
F = 220 C (A) ^{0.5}			<u><u>6,261 L/min</u></u>
Occupancy hazard reduction of surcharge		[yes/no]	
♦ Non-combustible	y		-25%
♦ Limited combustible			-15%
♦ Combustible			0%
♦ Free burning			15%
♦ Rapid burning			25%
			<u><u>4,696 L/min</u></u> (1)
Sprinkler Reduction			
♦ Non-combustible - Fire Resistive (3	no	50%	<u><u>0 L/min</u></u> (2)
Exposure surcharge (cumulative (%), 2 sides)		[yes/no]	
0 - 3 m	yes	25%	2 side 50%
3.1 - 10 m		20%	
10.1 - 20 m		15%	
20.1 - 30 m		10%	
30.1 - 45 m	yes	5%	2 side 10%
Cumulative Total			60%
			2,818 L/min
Fire Wall Separation		N/A	
♦ Number of Party Walls * 1000 L/min			
<i>(As per City of Ottawa Standard)</i>			<u><u>2,818 L/min</u></u> (3)
REQUIRED FIRE FLOW [(1) - (2) + (3)]			8,000 L/min
(2,000 L/min < Fire Flow < 45,000 L/min)			or 133.33 L/s
			or 1,761 IGPM
BY: Adam Lambros			
* Largest Size Unit			

Fire Flow Calculations - Single Residential Unit (3,000sqft +)

As per Fire Underwriter's Survey Guidelines

PROJECT: Area 6 Lands
JOB#: 113004

DATE: Aug 2015

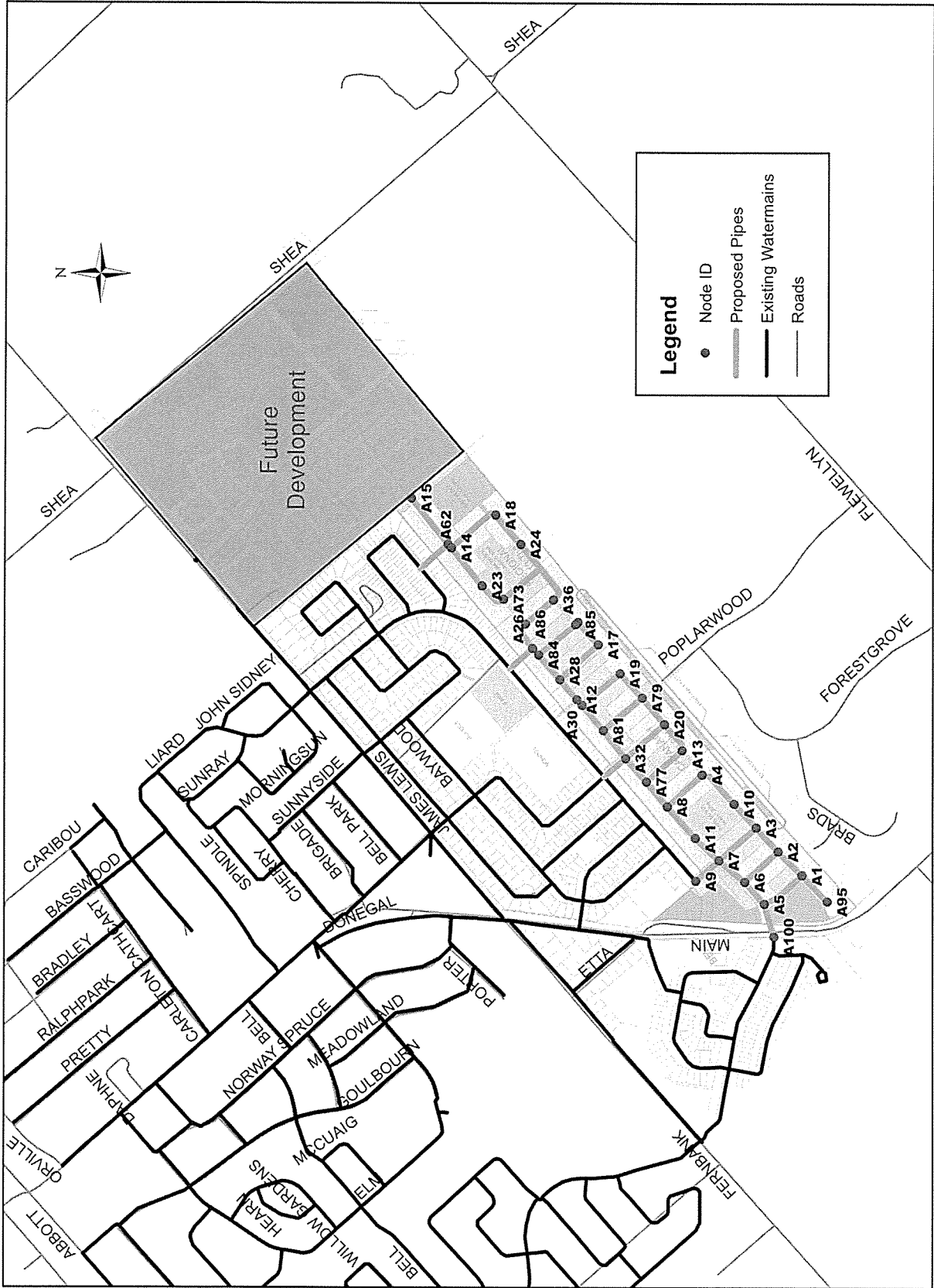
C Coefficient related to type of construction		[yes/no]	
♦ Wood frame	y		1.5
♦ Ordinary construction			1
♦ Non-combustible construction			0.8
♦ Fire resistive construction (< 2 hrs)			0.7
♦ Fire resistive construction (> 2 hrs)			0.6
♦ Interpolation (Using FUS Tables)			
A Area of structure considered (m²) *	360	<==>	3,875 ft²
<i>(All floors excluding Basement, under 2-Storeys)</i>			
F Required fire flow (L/min)			
F = 220 C (A) ^{0.5}			<u><u>6,261 L/min</u></u>
Occupancy hazard reduction of surcharge		[yes/no]	
♦ Non-combustible	y		-25%
♦ Limited combustible			-15%
♦ Combustible			0%
♦ Free burning			15%
♦ Rapid burning			25%
			<u><u>4,696 L/min</u></u> (1)
Sprinkler Reduction			
♦ Non-combustible - Fire Resistive (3)	no	50%	<u><u>0 L/min</u></u> (2)
Exposure surcharge (cumulative (%), 2 sides)		[yes/no]	
0 - 3 m	yes	25%	2 side 50%
3.1 - 10 m		20%	
10.1 - 20 m	yes	15%	1 side 15%
20.1 - 30 m	yes	10%	1 side 10%
30.1- 45 m		5%	
			Cumulative Total 75%
			3,522 L/min
Fire Wall Separation		N/A	
♦ Number of Party Walls * 1000 L/min			<u><u>3,522 L/min</u></u> (3)
<i>(As per City of Ottawa Standard)</i>			
REQUIRED FIRE FLOW [(1) - (2) + (3)]			8,000 L/min
(2,000 L/min < Fire Flow < 45,000 L/min)			or 133.33 L/s
			or 1,761 IGPM
BY: Adam Lambros			
* Largest Unit Size			

STITTSVILLE AREA 6 - POTABLE WATER HYDRAULIC ASSESSMENT OF PHASE 1 & 2

Appendix B Hydraulic Modelling Results
September 2, 2015

Appendix B HYDRAULIC MODELLING RESULTS

Figure B-1: Node IDs



PHASE 1

ID	BSDY				PKHR			
	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
A100	0.195	124.0	160.7	52	1.075	124.0	153.3	42
A6	0.195	122.2	160.7	55	1.075	122.2	153.3	44
A3	0.195	118.3	160.7	60	1.075	118.3	153.3	50
A10	0.385	117.4	160.7	62	0.385	117.4	153.3	51
A7	0.195	119.0	160.7	59	1.075	119.0	153.3	49
A11	0.385	116.8	160.7	62	0.385	116.8	153.3	52
A4	0.195	115.9	160.7	64	1.075	115.9	153.3	53
A13	0.195	113.5	160.7	67	1.075	113.5	153.3	57
A8	0.195	114.6	160.7	66	1.075	114.6	153.3	55
A77	0.195	113.5	160.7	67	1.075	113.5	153.3	57
A20	0.195	112.5	160.7	69	1.075	112.5	153.3	58
A79	0.195	112.6	160.7	68	1.075	112.6	153.3	58
A19	0.195	111.8	160.7	70	1.075	111.8	153.3	59
A32	0.195	112.9	160.7	68	1.075	112.9	153.3	57
A81	0.195	112.7	160.7	68	1.075	112.7	153.3	58
A30	0.195	111.7	160.7	70	1.075	111.7	153.3	59
A28	0.195	110.9	160.7	71	1.075	110.9	153.3	60
A17	0.195	110.5	160.7	71	1.075	110.5	153.3	61
A22	0.195	110.2	160.7	72	1.075	110.2	153.3	61
A26	0.195	111.8	160.7	70	1.075	111.8	153.3	59
A84	0.195	111.8	160.7	70	1.075	111.8	153.3	59
A85	0.195	110.2	160.7	72	1.075	110.2	153.3	61
A86	0.195	111.0	160.7	71	1.075	111.0	153.3	60
A36*	0.195	110.0	160.7	72	1.075	110.0	153.3	62
A12	0.195	111.7	160.7	70	1.075	111.7	153.3	59
A1	0.195	121.0	160.7	56	1.075	121.0	153.3	46
A2	0.195	119.0	160.7	59	1.075	119.0	153.3	49
A95**	0.195	125.0	160.7	51	1.075	125.0	153.3	40
A5	0.055	123.2	160.7	53	0.056	123.2	153.3	43
A9	0.055	119.1	160.7	59	0.056	119.1	153.3	49

*Node A36 is a connection to Phase 2; does remain a dead-end

**Node A95 is a dead-end located southwest of the development

**PHASE 1
MXDY+FF**

ID	Static Demand (L/s)	Static Pressure (psi)	Static Head (m)	Fire-Flow Demand (Lpm)	Residual Pressure (psi)	Available Flow at Hydrant (Lpm)	Available Flow Pressure (psi)
A1	0.489	49	155.4	10,000	23	13,000	20
A10	0.385	54	155.4	10,000	27	13,000	20
A100	0.489	45	155.4	10,000	24	13,000	20
A11	0.385	55	155.4	10,000	35	16,000	20
A12	0.489	62	155.4	10,000	42	17,000	20
A13	0.489	60	155.4	10,000	35	15,000	20
A17	0.489	64	155.4	10,000	33	13,000	20
A19	0.489	62	155.4	10,000	37	15,000	20
A2	0.489	52	155.4	10,000	29	15,000	20
A20	0.489	61	155.4	10,000	40	16,000	20
A22	0.489	64	155.4	10,000	33	13,000	20
A26	0.489	62	155.4	10,000	40	16,000	20
A28	0.489	63	155.4	10,000	42	17,000	20
A3	0.489	53	155.4	10,000	30	15,000	20
A30	0.489	62	155.4	10,000	42	18,000	20
A32	0.489	60	155.4	10,000	43	19,000	20
A36*	0.489	65	155.4	10,000	17	12,000	20
A4	0.489	56	155.4	10,000	29	13,000	20
A5	0.056	46	155.4	10,000	26	13,000	20
A6	0.489	47	155.4	10,000	27	14,000	20
A7	0.489	52	155.4	10,000	33	16,000	20
A77	0.489	60	155.4	10,000	41	18,000	20
A79	0.489	61	155.4	10,000	39	16,000	20
A8	0.489	58	155.4	10,000	39	17,000	20
A81	0.489	61	155.4	10,000	42	18,000	20
A84	0.489	62	155.4	10,000	40	16,000	20
A85	0.489	64	155.4	10,000	34	13,000	20
A86	0.489	63	155.4	10,000	36	14,000	20
A9	0.056	52	155.4	10,000	31	15,000	20
A95**	0.489	43	155.4	10,000	-2	8,000	20

*Node A36 is a connection to Phase 2; does remain a dead-end

**Node A95 is a dead-end located southwest of the development

PHASE 2

ID	BSDY				PKHR			
	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)	Demand (L/s)	Elevation (m)	Head (m)	Pressure (psi)
A100	0.195	124.0	160.7	52	1.075	124.0	152.9	41
A6	0.195	122.2	160.7	55	1.075	122.2	152.9	44
A3	0.195	118.3	160.7	60	1.075	118.3	152.9	49
A10	0.385	117.4	160.7	62	0.385	117.4	152.9	50
A7	0.195	119.0	160.7	59	1.075	119.0	152.9	48
A11	0.385	116.8	160.7	62	0.385	116.8	152.9	51
A4	0.195	115.9	160.7	64	1.075	115.9	152.9	53
A13	0.195	113.5	160.7	67	1.075	113.5	152.9	56
A8	0.195	114.6	160.7	66	1.075	114.6	152.9	54
A77	0.195	113.5	160.7	67	1.075	113.5	152.9	56
A20	0.195	112.5	160.7	69	1.075	112.5	152.9	57
A79	0.195	112.6	160.7	68	1.075	112.6	152.9	57
A19	0.195	111.8	160.7	69	1.075	111.8	152.9	58
A32	0.195	112.9	160.7	68	1.075	112.9	152.9	57
A81	0.195	112.7	160.7	68	1.075	112.7	152.9	57
A30	0.195	111.7	160.7	70	1.075	111.7	152.9	59
A28	0.195	110.9	160.7	71	1.075	110.9	152.9	60
A17	0.195	110.5	160.7	71	1.075	110.5	152.9	60
A22	0.195	110.2	160.7	72	1.075	110.2	152.9	61
A26	0.195	111.8	160.7	69	1.075	111.8	152.9	58
A84	0.195	111.8	160.7	69	1.075	111.8	152.9	58
A85	0.195	110.2	160.7	72	1.075	110.2	152.9	61
A86	0.195	111.0	160.7	71	1.075	111.0	152.9	59
A36	0.195	110.0	160.7	72	1.075	110.0	152.9	61
A12	0.195	111.7	160.7	70	1.075	111.7	152.9	59
A1	0.195	121.0	160.7	56	1.075	121.0	152.9	45
A2	0.195	119.0	160.7	59	1.075	119.0	152.9	48
A95**	0.195	125.0	160.7	51	1.075	125.0	152.9	40
A5	0.647	123.2	160.7	53	3.556	123.2	152.9	42
A9	0.647	119.1	160.7	59	3.556	119.1	152.9	48
A24	0.237	107.7	160.7	75	0.237	107.7	152.8	64
A73	0.264	110.6	160.7	71	1.454	110.6	152.8	60
A23	0.237	109.4	160.7	73	0.237	109.4	152.8	62
A18	0.264	107.0	160.7	76	1.454	107.0	152.8	65
A62	0.264	107.7	160.7	75	1.454	107.7	152.9	64
A14	0.264	108.9	160.7	74	1.454	108.9	152.9	62
A15	0	107.5	160.7	76	0.061	107.5	152.9	64
A21	0.264	109.6	160.7	73	1.454	109.6	152.8	61

**Node A95 is a dead-end located southwest of the development

**PHASE 2
MXDY+FF**

ID	Static Demand (L/s)	Static Pressure (psi)	Static Head (m)	Fire-Flow Demand (Lpm)	Residual Pressure (psi)	Available Flow at Hydrant (Lpm)	Available Flow Pressure (psi)
A1	0.489	49	155.3	10,000	30	13,000	20
A10	0.385	54	155.3	10,000	31	13,000	20
A100	0.489	44	155.3	10,000	27	12,000	20
A11	0.385	55	155.3	10,000	38	16,000	20
A12	0.489	62	155.2	10,000	45	17,000	20
A13	0.489	59	155.3	10,000	38	15,000	20
A14	0.661	66	155.2	10,000	42	15,000	20
A15	0	68	155.2	10,000	33	12,000	20
A17	0.489	64	155.2	10,000	37	13,000	20
A18	0.661	69	155.2	10,000	43	15,000	20
A19	0.489	62	155.2	10,000	40	15,000	20
A2	0.489	52	155.3	10,000	34	14,000	20
A20	0.489	61	155.3	10,000	43	16,000	20
A21	0.489	65	155.2	10,000	41	15,000	20
A22	0.237	64	155.2	10,000	37	13,000	20
A23	0.237	65	155.2	10,000	34	12,000	20
A24	0.489	68	155.2	10,000	42	15,000	20
A26	0.489	62	155.2	10,000	44	17,000	20
A28	0.489	63	155.2	10,000	45	17,000	20
A3	0.489	52	155.3	10,000	35	15,000	20
A30	0.489	62	155.2	10,000	45	18,000	20
A32	0.489	60	155.3	10,000	46	19,000	20
A36	0.489	64	155.2	10,000	42	15,000	20
A4	1.616	56	155.3	10,000	32	13,000	20
A5	0.489	46	155.3	10,000	29	13,000	20
A6	0.661	47	155.3	10,000	30	13,000	20
A62	0.489	68	155.2	10,000	44	15,000	20
A7	0.661	52	155.3	10,000	36	15,000	20
A73	0.489	63	155.2	10,000	32	12,000	20
A77	0.489	59	155.3	10,000	44	18,000	20
A79	0.489	61	155.3	10,000	42	16,000	20
A8	0.489	58	155.3	10,000	42	17,000	20
A81	0.489	61	155.3	10,000	45	18,000	20
A84	0.489	62	155.2	10,000	44	17,000	20
A85	0.489	64	155.2	10,000	37	13,000	20
A86	1.616	63	155.2	10,000	43	16,000	20
A9	0.489	51	155.3	10,000	34	14,000	20
A95**	0.489	43	155.3	10,000	5.1	8,000	20

**Node A95 is a dead-end located southwest of the development

2 WATER SUPPLY

2.1 Existing Conditions

The proposed development is located within the service area of Pressure Zone 3W of the City of Ottawa water distribution system. The zone is fed by the Glen Cairn and Campeau Drive Pump Stations, both of which are remote from the site. Balancing storage during peak and fire flow conditions is provided by the Stittsville Elevated Tank. There are several existing watermains adjacent to the site including 200 mm diameter watermains on both Fernbank Road and Friendly Crescent and a 200 mm diameter watermain in Fernbank Road, west of the site. As part of the development of the adjacent Stittsville South lands a 250 mm watermain will be extended along Hickstead Drive which extends to Street No. 3 in Phase 1. **Figure 2.1** shows the location of the existing Water Plan adjacent to the site.

2.2 Serviceability Study

A conceptual water plan for the Stittsville South Area 6 area was included in the 2013 MSR study. A copy of the recommended plan, Watermain Concept Plan – Figure 6.1 from that report is included in **Appendix A**. The main elements of the recommended plan for the subject site include an extension of the proposed 250 mm diameter main spine through the subject site from the west (Regional Lands) and connecting to the existing watermain on Fernbank Road to the north.

2.3 Design Criteria

2.3.1 Water Demands

Water demands have been calculated for the full development including Phase 1. Per unit population density and consumption rates are taken from Tables 4.1 and 4.2 at the Ottawa Design Guidelines – Water Distribution and are summarized as follows:

- | | |
|----------------------------------|------------------------|
| • Single Family | 3.4 person per unit |
| • Townhouse and Semi-Detached | 2.7 person per unit |
| • Average Apartment | 1.8 person per unit |
| • Residential Average Day Demand | 350 l/cap/day |
| • Residential Peak Daily Demand | 875 l/cap/day |
| • Residential Peak Hour Demand | 1,925 l/cap/day |
| • ICI Average Day Demand | 50,000 l/gross ha/day |
| • ICI Peak Daily Demand | 75,000 l/gross ha/day |
| • ICI Peak Hour Demand | 135,000 l/gross ha/day |

Residential units in Phase 1 consist of single family, semi-detached and street townhouses. A future commercial site which is not part of this development is located at the north corner of the site adjacent to Fernbank and Shea Roads, the water demands for this site is included in the design. A watermain demand calculation sheet is included in **Appendix A** and the total water demands are summarized as follows:

	<u>Full Development</u>	<u>Phase 1</u>
• Average Day	10.85 l/s	6.39 l/s
• Maximum Day	25.53 l/s	14.39 l/s
• Peak Hour	55.25 l/s	30.76 l/s

2.3.2 System Pressure

The Ottawa Design Guidelines – Water Distribution (WDG001), July 2010, City of Ottawa, Clause 4.2.2 states that the preferred practice for design of a new distribution system is to have normal operating pressures range between 345 kPa (50 psi) and 552 kPa (80 psi) under maximum daily flow conditions. Other pressure criteria identified in Clause 4.2.2 of the guidelines are as follows:

Minimum Pressure	Minimum system pressure under peak hour demand conditions shall not be less than 276 kPa (40 psi)
Fire Flow	During the period of maximum day demand, the system pressure shall not be less than 140 kPa (20 psi) during a fire flow event.
Maximum Pressure	Maximum pressure at any point in the distribution system shall not exceed 689 kPa (100 psi). In accordance with the Ontario Building/Plumbing Code, the maximum pressure should not exceed 552 kPa (80 psi). Pressure reduction controls will be required for buildings where it is not possible/feasible to maintain the system pressure below 552 kPa.

2.3.3 Fire Flow Rates

The Fire Underwriters Survey (FUS) method of calculating fire flow requirements is to be used in accordance with the Ottawa Design Guidelines – Water Distribution. In the FUS method, wood frame buildings with separations less than three meters are considered one fire area. Buildings in the Davidson Lands Development are wood frame buildings, with separation less than three meters. An example is on Street No. 11 between lots 73 and 81 in which the 9 single family lots all have separations less than three meters thus making one fire area. A FUS calculation for this area is included in **Appendix A**. The calculations predict that the fire flow requirement needs to be 22,000 l/min which is impractical to supply with local water mains.

In the recent Technical Bulletin 'ISDTB-2014-02, Revisions to Ottawa Design Guidelines – Water', the fire flow requirements for single detached dwellings and traditional town and row houses can be capped at 10,000 l/min provided that there is a minimum separation of 10 meters between the backs of adjacent units and that the town and row house blocks are limited to 600 square meters of building areas and seven dwelling units. Since the residential units in the Davidson Lands meet the requirements of ISDTB-2014-02, the fire flow rate of 10,000 l/min (166.7 l/s) is used in the fire flow analysis.

There are no details for the future commercial site at the north of the development. Since the site is bisected by the Hydro One corridor there is a limit on the size of the building that can be placed on the site therefore a fire flow rate of 12,000 l/min (200 l/s) for the external commercial development is used in our fire flow analysis.

2.3.4 Boundary Conditions

The City of Ottawa has provided hydraulic boundary conditions at two locations; one at the existing main on Fernbank Road at Hartsmere Drive and the other at the southwest intersection of Friendly Crescent and Hartsmere Drive. Two separate conditions were given for the max day plus fire

scenario, one for the 167 l/s residential fire flow and a separate one for the 200 l/s commercial fire flow. A copy of the boundary conditions is included in **Appendix A** and summarized as follows:

	CONNECTION 1 FERNBANK ROAD	CONNECTION 2 FRIENDLY CRESCENT
Max HGL (Basic Day)	160.4 m	160.4 m
Peak Hour	151.1 m	150.8 m
Max Day + Fire (167 l/s Fire Flow)	142.3 m	135.2 m
Max Day + Fire (200 l/s Fire Flow)	137.5m	127.8 m

2.3.5 Hydraulic Model

A computer model for the overall Davidson Lands along with a separate model 1 containing only Phase 1 has been developed using the H2O MAP Version 6.0 program produced by MWH Soft Inc. The model includes the existing watermains and boundary conditions at Fernbank Road and Friendly Crescent.

2.4 Proposed Water Plan

2.4.1 Modeling Results

The hydraulic model was run under basic day, maximum day with fire flows and under peak hour conditions for the overall development and Phase 1 only. Water pipes are sized to provide sufficient pressure and to deliver the required fire flows. During the design stage all mains are tested at the minimum 150 mm diameter size, while the pressure criteria is met with the minimum sized mains the fire flow requirement is not achieved at all locations. The main sizes are increased in an iterative process until the fire flow results are sufficient for both the overall sub-division and Phase 1.

Results of the hydraulic model are include in **Appendix A** and summarized as follows:

<u>Scenario</u>	<u>Overall</u>	<u>Phase 1 Only</u>
Basic Day (Max HGL) Pressure Range	454.5 to 534.8 kPa	459.5 to 535.9 kPa
Peak Hour Pressure Range	359.6 to 438.4 kPa	367.1 to 440.9 kPa
Max Day + 167 l/s Fire Flow Minimum Flow	163.9 l/s	150.5 l/s
Max Day + 200 l/s Fire Flow Minimum Flow	247.3 l/s	208.8 l/s

A comparison of the results and design criteria is summarized as follows:

Maximum Pressure	All nodes in both analysis have basic day pressures under 552 kPa, therefore pressure reducing control is not required for this development.
Minimum Pressure	All nodes in the model exceed the minimum value of 276 kPa (40 psi).

Fire Flow All residential nodes exceed the fire flow requirement of 166.7 l/s in the overall development. The fire flow for the commercial site exceeds the 200 l/s requirement using the boundary condition for the 200 l/s flow.

Under the Phase 1 analysis, there is one location at Node J24 at the south intersection of Maygrass Way and Kayenta Street where the fire flow is less than the requirement. The Phase 1 fire flow at Node 24 is 150.5 l/s which is within 90% of the 166.7 l/s requirement, the fire flow at this node increases to 204.0 l/s when the adjacent Phase 2 is constructed.

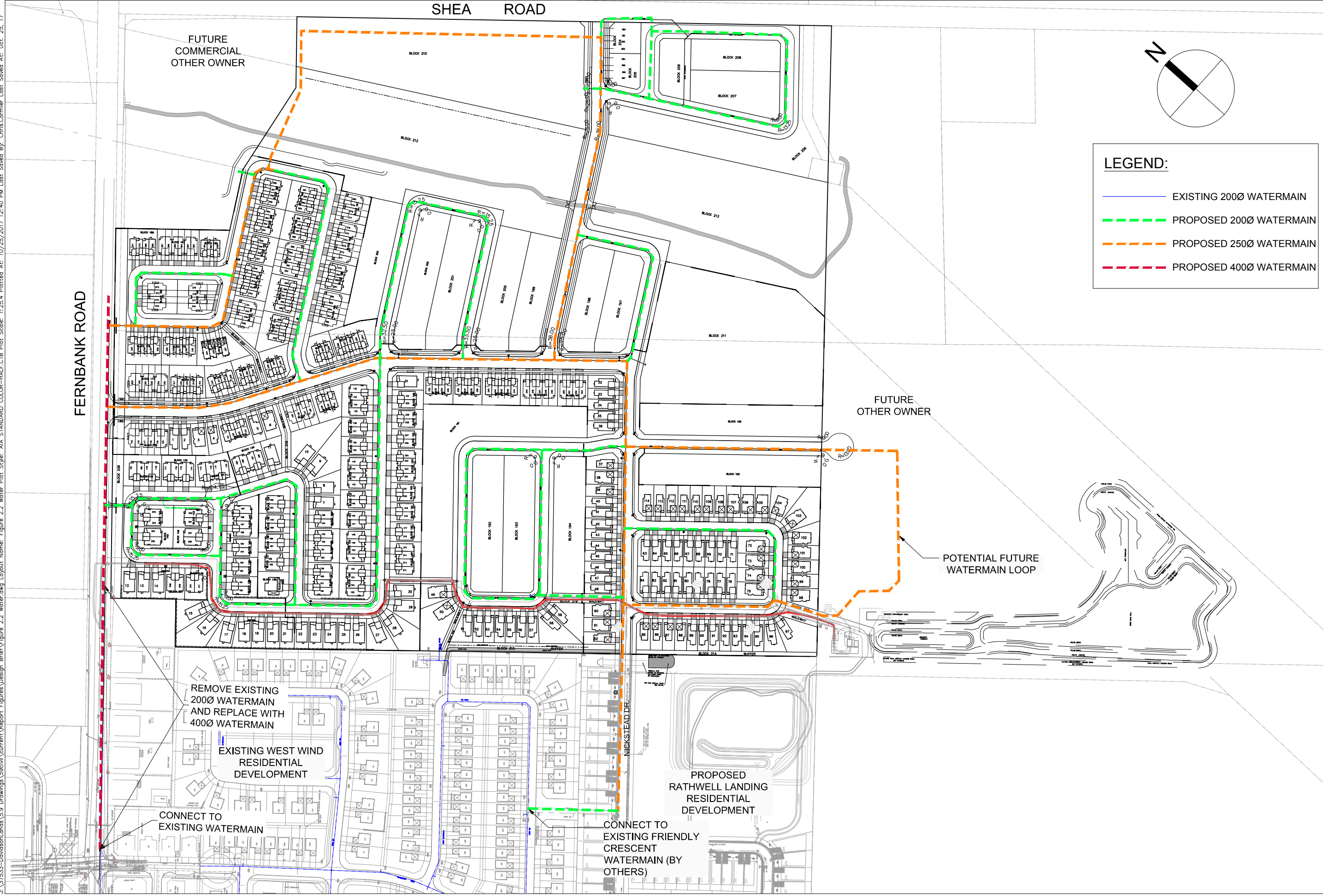
2.4.2 Watermain Layout

Figure 2.2 shows the proposed Water Plan for both Phase 1 and the balance of the sub-division.

In the 2013 MSR, a 400 mm watermain is proposed on Fernbank Road along the frontage of the Davidson Lands connecting to an existing 200 mm watermain that is shown on Figure 2.1. With the boundary conditions provided at Fernbank and Hartsmere Drive as shown in Section 2.3.4., the fire flows in the site range from 99.4 l/s to 146.4 l/s, in order to achieve the required fire flows the existing 200 mm watermain is required to be replaced and the 400 mm watermain will be extended to Hartsmere Drive.

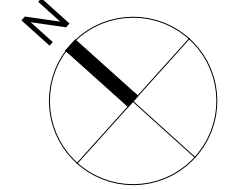
A 250 mm watermain will be extended from the adjacent Stittsville South development along Hickstead Way and Edenwyld Drive to connect to the 400 mm main on Fernbank Road. For Phase 1 a second connection is required to the 400 mm main on Fernbank Road that will extend through Kayenta Street which is part of Phase 2. In order to service future Phases 2 and 3, a 250 mm watermain is required to connect to the 400 mm watermain on Fernbank Road at Jardiniere Street, this 250 mm main will, in a future phase cross the hydro corridor and a 250 mm main will be extended along Cosanti Drive connecting to the 250 mm main on Edenwyld Drive.

J:\37533-Davidson\Landis\5.9 Drawings\5901\Current\Report Figures\Design Brief\Figure 2.2 Water.dwg Layout Name: Figure 2.2 Water Plot Style: AIA STANDARD COLOR-HALF.CTB Plot Scale: 1:25.4 Plotted At: 10/25/2017 12:40 PM Last Saved By: Chris.Cornier Last Saved At: Oct. 25, 17



LEGEND:

- EXISTING 2000Ø WATERMAIN
- - - PROPOSED 2000Ø WATERMAIN
- - - PROPOSED 2500Ø WATERMAIN
- - - PROPOSED 4000Ø WATERMAIN



Sheet No.

Drawing Title

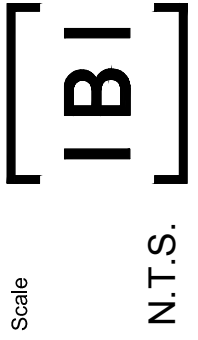
Project Title

Scale

FIGURE 2.2

PROPOSED WATER PLAN

DESIGN BRIEF
DAVIDSON LANDS-OPA 76 AREA 6a
PHASE 1
STITTSVILLE SOUTH



APPENDIX D

STORMWATER

January 30, 2023

Project Number: P2267

David Schaeffer Engineering Limited
120 Iber Road, Unit 103
Stittsville, ON
K2S 1E9

Attention: Kevin Murphy, P.Eng.

**Subject: Caivan – Stittsville Lands (5993, 6070 & 6115 Flewellyn Road):
Pre-Development Hydraulic and Hydrologic Study**

Introduction

As set out in the Terms of References for the Caivan Stittsville Lands (5993, 6070 & 6115 Flewellyn Road), drafted by David Schaeffer Engineering Ltd (DSEL) on June 9, 2022, the pre-development hydraulic and hydrologic conditions of the proposed development site are required to be assessed. The following memo will assess the existing major flow patterns within and around the site, and outline the findings of a detailed pre-development water budget analysis based on hydrologic modelling using site-based soil data and historical rainfall data.

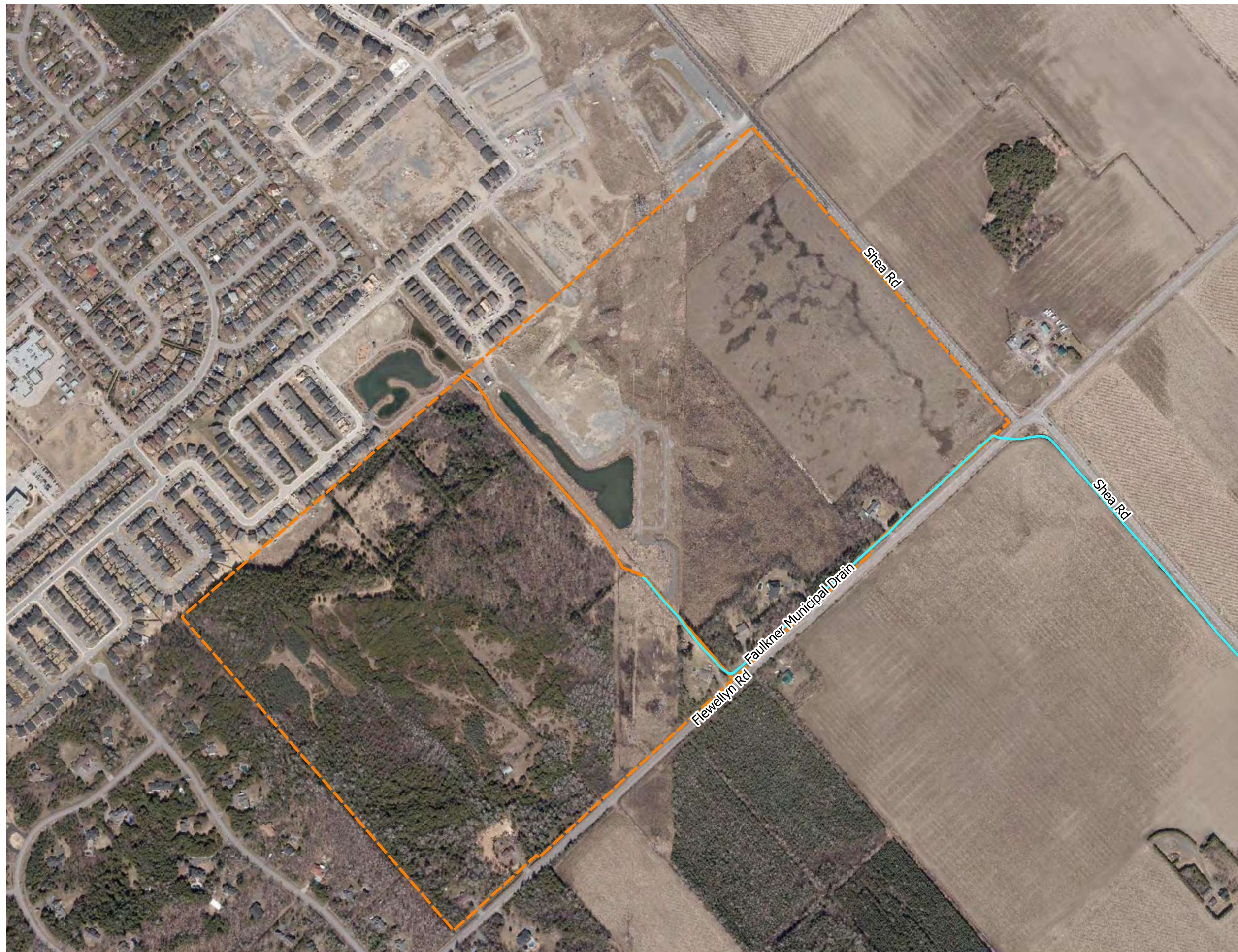
Site Overview

The subject lands are bound by Flewellyn Road to the south, Shea Road to the east, an existing urban subdivision development to the north (Stittsville South – Area 6) and an estate lot subdivision to the west. The area is also bisected by the Faulkner Municipal Drain and a Hydro One power transmission corridor. For this study, the overall development area has been broken into two sections (referred to as east and west) bisected by the municipal drain and hydro corridor. The property parcel of 5993 Flewellyn Road (east) is cleared of trees and vegetation, while the west parcels (comprising 6070 & 6115 Flewellyn Road) are treed with patches of grassed areas. **Figure 1** provides an overview of the development site relative to the Faulkner drain and major roads.



Pre-Development Drainage

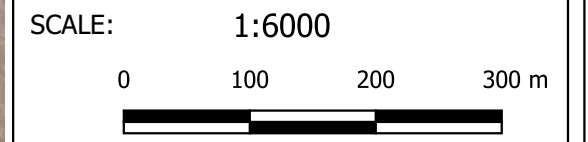
1m LiDAR flown in 2020 by the City of Ottawa has been obtained to determine the existing primary flow patterns within the site. This topographic data was imported into GIS software with watershed delineation tools applied to determine the drainage areas and primary flow paths within the site. **Figure 2** provides an overview of the primary existing subwatersheds and flow paths within the site.

From this analysis, it is seen that for the eastern lands, the site primarily consists of 2 major drainage areas both of which discharge to the Faulkner Drain on Flewellyn Road. For the west property, the drainage patterns are slightly more complex but approximately half of the lands discharge to the Faulkner Drain on Flewellyn road, while the remaining half discharges to the Faulkner drain which divides the east and west properties. Note that there is no external drainage area that flows across either site.



Legend

-  Development Area
-  Municipal Drain



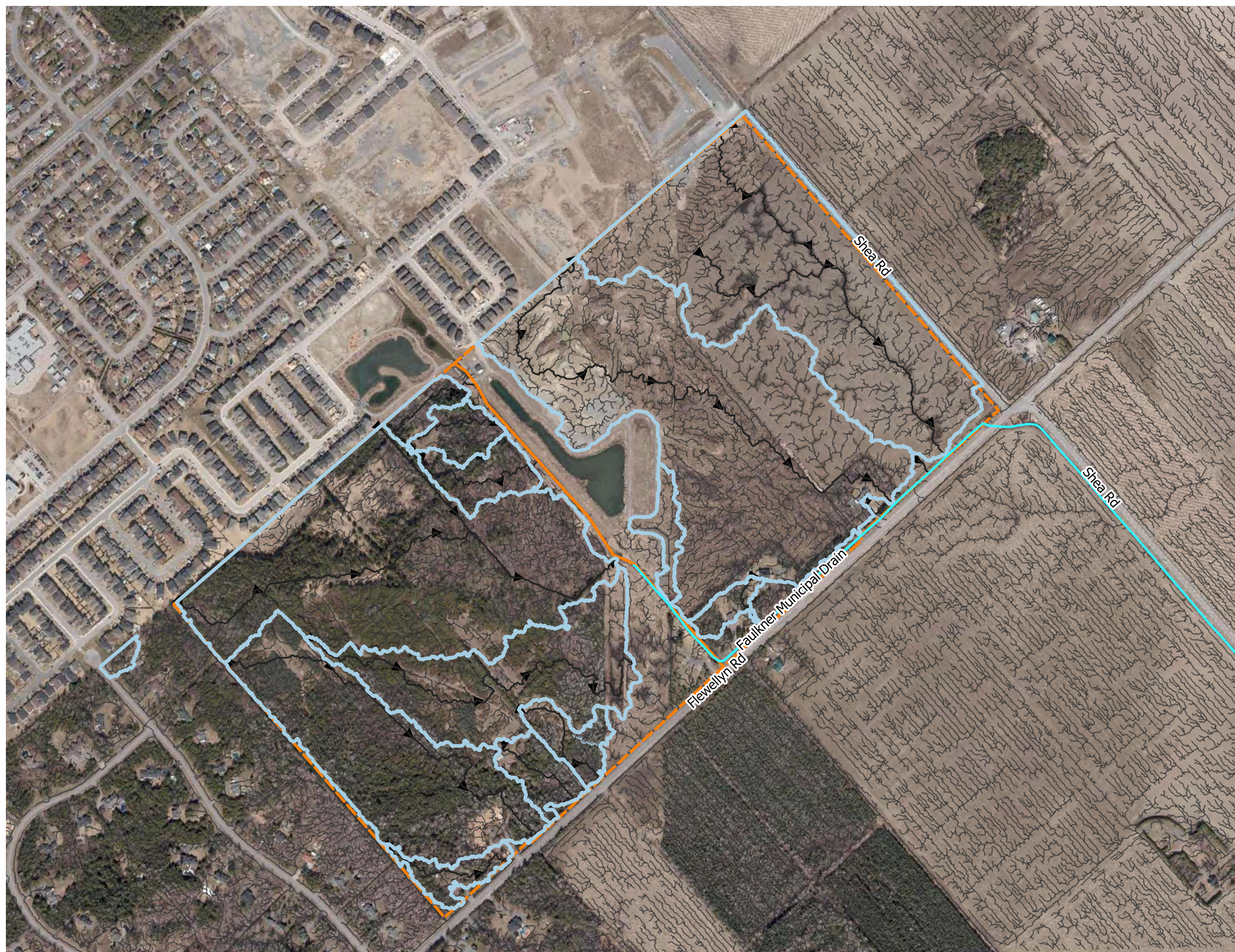
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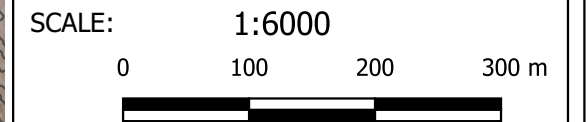
Figure 1: Site Overview


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DATE	JAN 2023



Legend

-  Drainage Areas
-  Development Area
-  Municipal Drain
-  Primary Flow Paths
-  Drainage Pattern



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Figure 2: Existing Drainage Patterns

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Pre-Development Water Budget Analysis

A continuous SWMHYMO model has been developed to assess the site's pre-development water budget. This model makes use of site-based infiltration testing results as well as local climatic conditions, the following sections outline each of these items in detail.

Soil Infiltration / CN

Paterson Group completed Guelph Permeameter testing throughout the site, to determine the site's soil infiltration rates. From this analysis, soils were tested at both shallow depths (approximately 0.3 m - 0.5 m below ground) and deeper depths (0.5 m >). Based on Paterson's site testing the majority of the site consist of Brown Silty Sand to Sandy Silt with some localized pockets of silty clay with sand. Overall the site was found to have soil infiltration rates in the range of **26 mm/hr** to **76 mm/hr**. The localized infiltration results of this testing have been mapped in GIS and an inverse distance weighting algorithm was applied to provide a complete infiltration map of the site, **Figure 3** provides an overview of this mapping. A full summary of Paterson's site infiltration testing can be found in **Attachment A**. Based on Paterson's Site investigation the soils present are considered a "Type C" hydrologic soil group.

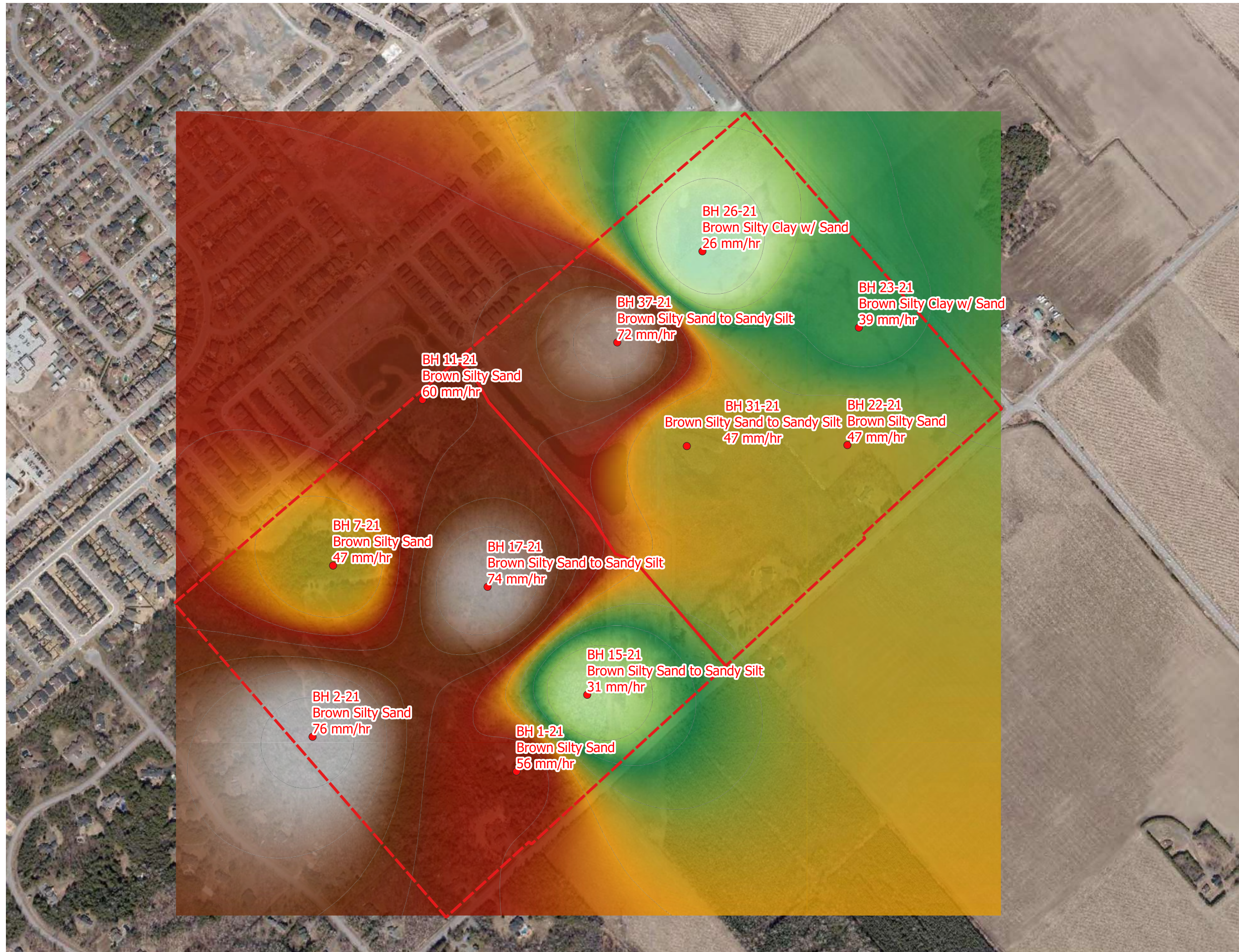
The latest available (2021) aerial mapping for the City of Ottawa was used to discretize the various land use conditions throughout the development site. **Figure B1** in **Attachment B** provides a visual overview of the study area. This land use data was merged with the underlying soil types to derive a Curve Number (CN), based on applicable values outlined in Tables A2 and A3 in the SWMHYMO Manual. Each Curve Number was then weighted based on the total area within a given subcatchment to determine the weighted CN for that subcatchment, see **Table B1** in **Attachment B**. Based on this analysis the site has a CN* of **65** and **61** for the lands East and West of the Faulkner Drain respectively.

Time to Peak

The time-to-peak values have been calculated based on existing topography using the City of Ottawa LiDAR. Flow paths have been discretized based on the topographic data using GIS tools and the longest major flow path was identified; **Figure B2** in **Attachment B** outlines the flow path discretization. The upstream and downstream topographic elevations and flow lengths were identified and used in the calculations. For this natural subcatchment, the Federal Aviation Administration (FAA) method was determined to be the most appropriate method to calculate the Time to Peak. **Table B2** in **Attachment B** provides full details of these calculations, along with other time-to-peak values using alternative t_p calculation methods.

Continuous Simulations

A continuous SWMHYMO model was developed to assess the site's water budget under pre-development conditions. This model was run using 36 years of hourly rainfall data from the Ottawa International Airport from 1967 to 2003 (excluding missing 2001 rainfall data), the average annual evaporation, infiltration and runoff volumes from the subject site were computed and compared. Note that this rain gauge is generally only operational for the months of April-November. Outside of this window precipitation is more likely to be in the form of snowfall and the soils are also more likely to be frozen, making it difficult to simulate such conditions with a hydrologic model using conventional City parameters, as such, this period has not been considered in the analysis.

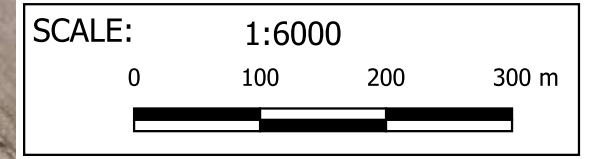


Legend

- Development Area
- Infiltration Locations

Infiltration (mm/hr)

- 25.00
- 30.00
- 35.00
- 40.00
- 45.00
- 50.00
- 55.00
- 60.00
- 65.00
- 70.00
- 75.00
- 80.00



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Figure 3: Infiltration

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Simulation Results

The complete SWMHYMO modelling input and output files have been provided in **Attachment B. Table B3** provides the full summary of the SWMHYMO modelling, based on the 39 years of data, and outlines the maximum, minimum and average volumes and percentages of precipitation that evaporate infiltrate and runs off the site, **Table 1** below is an excerpt this summary.

Table 1: Pre-Development Water Budget based on Continuous Simulations

Precipitation (mm)	Evaporation		Infiltration		Runoff	
	(mm)	(%)	(mm)	(%)	(mm)	(%)
589.1	370.7	63%	116.9	20%	101.6	17%

Based on the continuous simulations using 39 years of historical rainfall data it was determined that for the total development site, approximately **17%** of the annual rainfall will result in runoff, **63%** will evaporate and **20%** will infiltrate.

Existing Hydraulic Conditions

Robinson Consulting Inc completed an updated hydraulic model of the Faulkner Drain as a part of their December 2020 “*Amendment to the Engineer’s Report for the Faulkner Municipal Drain*”. As a part of this study, modifications were proposed to the existing Faulkner Municipal Drain which included relocating a portion of the drain, lowering the drain profile, and modifying the cross-section of the drain to increase the capacity and reduce the potential for erosion of the steep banks. This study also considered the upgrading/replacement of 3 culverts within the drain. At the time of drafting this memo, these updates are either completed or currently under construction, as such can be reflective of current conditions.

Based on Table 4.2 of the Robinsons report there are three existing culverts that act as residential entrances on Flewellyn Road (Culverts 4+882.90, 5+055.00 & 5+185.40) that are either close to or have slightly less than the required capacity to safely convey the full 100-year flow. The culverts are likely controlling water levels along this portion of Flewellyn Road, as such these culverts should be revisited in the future to ensure that peak water levels are contained within the Faulkner Municipal drain.

Conclusion

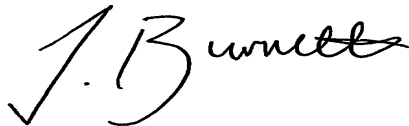
In summary, a detailed topographic study has been undertaken on the site to identify all major flow paths within the development under existing conditions. From this analysis, it was found that for the eastern lands, the site primarily consists of 2 major drainage areas both of which discharge to the Faulkner Drain on Flewellyn Road. For the western lands, the drainage patterns are slightly more complex but approximately half of the lands discharge to the Faulkner Drain on Flewellyn Road, while the remaining half discharges to the Faulkner Drain where it divides the east and west properties.

Continuous hydrologic modelling has been completed which has made use of soil infiltration testing completed by Paterson Group to determine the site's predevelopment water budget. Based on this analysis it was determined that for the total development site, approximately **17%** of the annual rainfall will result in runoff, **63%** will evaporate and **20%** will infiltrate.

Based on Table 4.2 of the Robinsons report there are three existing culverts that act as residential entrances on Flewellyn Road (Culverts 4+882.90, 5+055.00 & 5+185.40) that are either close to or have slightly less than the required capacity to safely convey the full 100-year flow. The culverts are likely controlling water levels along this portion of Flewellyn Road, as such these culverts should be revisited in the future to ensure that peak water levels are contained within the Faulkner Municipal drain.

Yours truly,

J.F Sabourin and Associates Inc.



Jonathon Burnett, B.Eng, P.Eng
Water Resources Engineer



cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects

Figures

- Figure 1: Site Overview
- Figure 2: Existing Drainage Patterns
- Figure 3: Soil Infiltration Map

Tables

- Table 1A: Existing Water Budget Summary

Attachments

- Attachment A: Paterson Group Soil Infiltration Testing
- Attachment B: Water Budget Analysis



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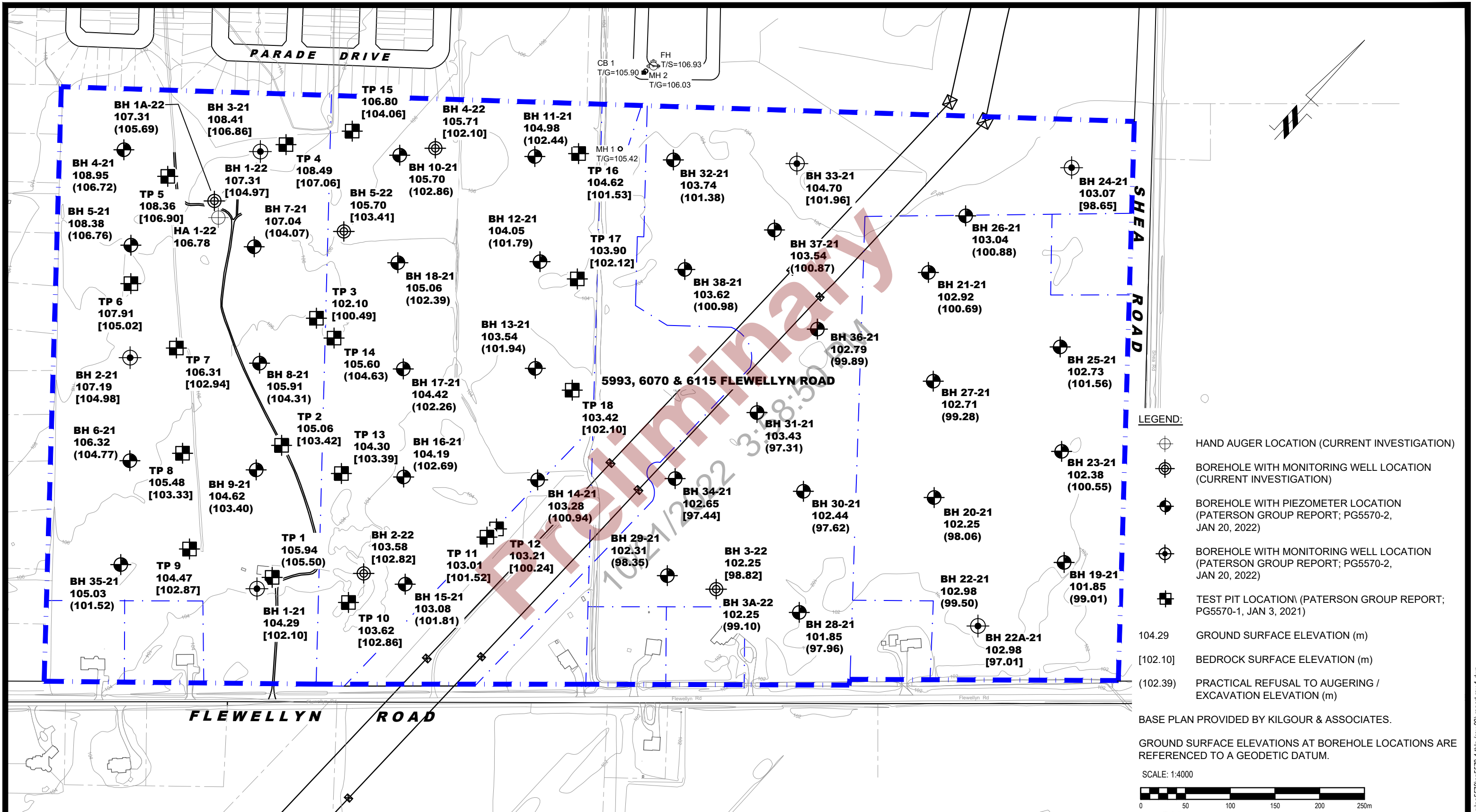
Ottawa, ON
Paris, ON
Gatineau, QC
Montréal, QC
Québec, QC

Attachment A

Paterson Group Soil infiltration Testing

Table 1 - Overburden Field Saturated Hydraulic Conductivity Results and Estimated Infiltration Rates				
Test Completed Adjacent to Borehole ID	Infiltration Testing Elevation (m asl)	Material	K_{fs} (m/s)	Unfactored Infiltration Rate (mm/hr)*
BH1-21	103.90	Brown Silty Sand	2.10E-06	56
	103.63	Brown Silty Sand	1.90E-06	56
BH2-21	106.95	Brown Silty Sand	6.40E-06	76
	106.65	Brown Silty Sand	5.30E-07	39
BH7-21	106.74	Brown Silty Sand	1.10E-06	47
	106.44	Brown Silty Sand	1.60E-06	52
BH11-21	104.68	Brown Silty Sand	2.70E-06	60
	104.38	Brown Silty Sand	1.60E-06	52
BH15-21	102.70	Brown Silty Sand to Sandy Silt	2.10E-07	31
	102.48	Brown Silty Sand to Sandy Silt	≤ 8.1E-09	≤ 13
BH17-21	106.74	Brown Silty Sand to Sandy Silt	5.90E-06	74
	106.44	Brown Silty Sand to Sandy Silt	4.10E-06	67
BH22-21	102.58	Brown Silty Sand	1.10E-06	47
	102.28	Brown Silty Sand	1.60E-06	52
BH23-21	102.33	Brown Silty Clay w/ Sand	5.30E-07	39
	101.70	Brown Silty Clay	≤ 8.1E-09	≤ 13
BH26-21	102.74	Brown Silty Clay w/ Sand	1.10E-07	26
	102.44	Brown Silty Clay w/ Sand	1.10E-07	26
BH29-21	101.87	Brown Silty Sand to Sandy Silt	5.30E-07	39
	101.57	Brown Silty Sand to Sandy Silt	2.70E-07	33
BH31-21	103.19	Brown Silty Sand to Sandy Silt	1.10E-06	47
	102.89	Brown Silty Sand to Sandy Silt	1.35E-07	27
BH37-21	103.21	Brown Silty Sand to Sandy Silt	5.30E-06	72
	102.91	Brown Silty Sand to Sandy Silt	5.90E-06	74

*The infiltration rates do not include a safety correction factor. Based on our testing results, a safety correction factor can range between 2.5 to ≥ 3.5.



- LEGEND:**
- HAND AUGER LOCATION (CURRENT INVESTIGATION)
 - BOREHOLE WITH MONITORING WELL LOCATION (CURRENT INVESTIGATION)
 - BOREHOLE WITH PIEZOMETER LOCATION (PATERSON GROUP REPORT; PG5570-2, JAN 20, 2022)
 - BOREHOLE WITH MONITORING WELL LOCATION (PATERSON GROUP REPORT; PG5570-2, JAN 20, 2022)
 - TEST PIT LOCATION (PATERSON GROUP REPORT; PG5570-1, JAN 3, 2021)
 - 104.29 GROUND SURFACE ELEVATION (m)
 - [102.10] BEDROCK SURFACE ELEVATION (m)
 - (102.39) PRACTICAL REFUSAL TO AUGERING / EXCAVATION ELEVATION (m)

BASE PLAN PROVIDED BY KILGOUR & ASSOCIATES.

GROUND SURFACE ELEVATIONS AT BOREHOLE LOCATIONS ARE REFERENCED TO A GEODETIC DATUM.

SCALE: 1:4000



NO.	REVISIONS	DATE	INITIAL
2	BH 1-22 - BH 5-22 & HA 1-22 ADDED TO PLAN	10/03/2022	KP
1	BH 1-21 - BH 38-21 ADDED TO PLAN	01/20/2022	OC

CAIVAN COMMUNITIES
GEOTECHNICAL INVESTIGATION
PROPOSED RESIDENTIAL DEVELOPMENT
5993, 6070 & 6115 FLEWELLYN ROAD

OTTAWA, ONTARIO

TEST HOLE LOCATION PLAN

Scale:	1:4000	Date:	01/2022
Drawn by:	JM	Report No.:	PG5570-2, REVISION 1
Checked by:	KP	Dwg. No.:	PG5570-1
Approved by:	DJG	Revision No.:	2

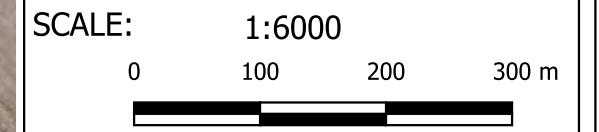
Attachment B

Water Budget Analysis



Legend

- Development Area
- Impervious (SWM)
- Mature Forest
- Pasture and Shrubs
- Urban Lawn/
Shallow Rooted Crops



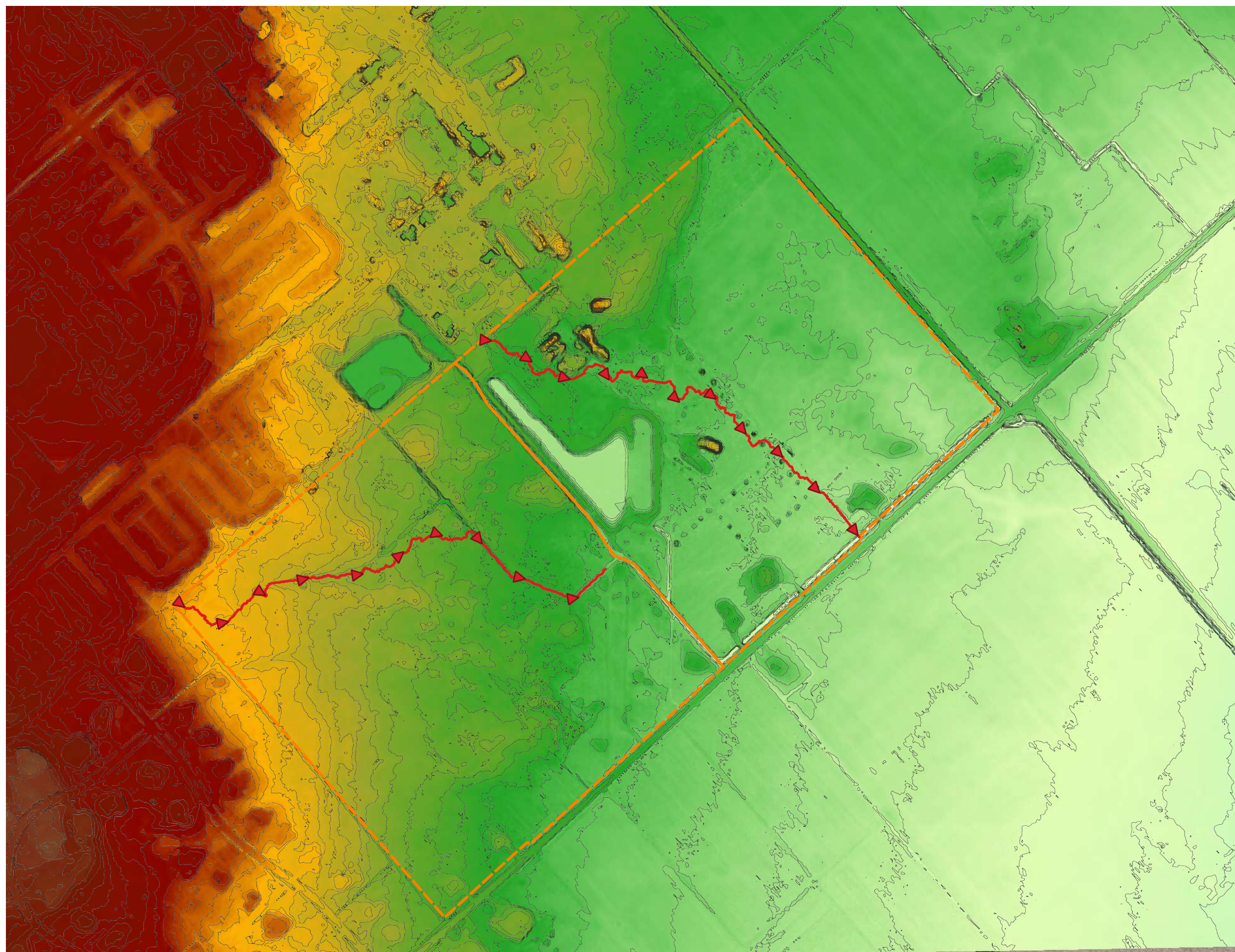
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Figure B1: Land Use

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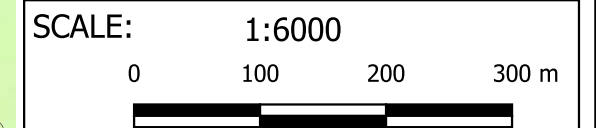
Legend


 Development Area

 Flow Lengths

Terrain (m)

-  98
-  100
-  103
-  106
-  109
-  112
-  115
-  118
-  121
-  124



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Figure B2: Flow Lengths

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Table B1: Calculation of SCS Curve Number (CN) and Modified Curve Number (CN*)

EAST (39.345 ha)							
Area (ha)	Land Type	Soil Name	Soil		CN	% of Catchment	Weighted CN
			Condition	Soil Group			
15.625	Urban Lawn/Shallow Rooted Crops	Fine Sandy Loam	C	Fair	79	39.7%	31.4
22.565	Pasture and Shrubs	Fine Sandy Loam	C	Fair	70	57.4%	40.1
1.155	Impervious (SWM)	Fine Sandy Loam	C	Fair	99	2.9%	2.9
						CN	74.4
						CN*	65

WEST (39.847 ha)							
Area (ha)	Land Type	Soil Name	Soil		CN	% of Catchment	Weighted CN
			Condition	Soil Group			
24.901	Mature Forest	Fine Sandy Loam	C	Fair	73	62.5%	45.6
14.946	Pasture and Shrubs	Fine Sandy Loam	C	Fair	70	37.5%	26.3
						CN	71.9
						CN*	61

Table B2: Time to Peak Calculations

Parameter	Units	East	West
Area	ha	39.35	39.85
CN*	-	65	61
Ptotal to calc C from CN, use 2 yr 3 hr Chicago stom	P(mm)	33.2	33.2
	Ia(mm)	4.67	4.67
	RV(mm)	4.8	4.3
Ptotal to calc C from CN, use 2 yr 24 hr SCS stom	P(mm)	52.77	52.77
	RV(mm)	12.3	11.0
C (From Chicago storm)	-	0.15	0.13
C (From SCS storm)	-	0.23	0.21
Length of Channel	m	1012	976
	ft	3320	3201
Elevation of Head Water	m	104.35	109.56
	ft	342	359
Elevation of Outlet	m	100.16	102.45
	ft	329	336
Average Slope	m/m	0.41%	0.73%
	ft/ft	0.41%	0.73%
Kirpich			
Time of Concentration	mins	33	26
Time to Peak	min	22	17
Time to Peak	Hours	0.37	0.29
FAA (From Chicago storm)			
Time of Concentration	mins	133	110
Time to Peak	mins	88	73
Time to Peak	Hours	1.47	1.22
FAA (From SCS storm)			
Time of Concentration	mins	121	101
Time to Peak	mins	80	67
Time to Peak	Hours	1.34	1.12
Barnsby Williams			
Time of Concentration	mins	48	41
Time to Peak	mins	32	28
Time to Peak	Hours	0.53	0.46
SCS			
Time of Concentration	mins	199	159
Time to Peak	mins	133	106
Time to Peak	Hours	2.21	1.77
Selected Method			
FAA (From Chicago storm)			
Time to Peak	min	88	73
Time to Peak	Hours	1.47	1.22

Note:

All methods calculated as per Appendix A of the SWMHYMO manual

Time to Peak calculated as 2/3 Time of concentration

```

1  20      Metric units / ID Numbers OFF
2  *#*****
3  *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
4  *#*****
5  *# Project Name : [Caivan Stittsville West properties]
6  *# Project Number: [2267]
7  *# Date : [2021/12/14]
8  *# Modeller : [JB]
9  *# Company : J.F. Sabourin and Associates
10 *# License # : 2549237
11 *#*****
12 *#*****
13 START          TZERO=[1967.0101], METOUT=[2], NSTORM=[0], NRUN=[67]
14 *%          [""] <--storm filename, one per line for NSTORM time
15 *%-----|-----|
16 *# Ottawa International Airport (1967 - 2003)
17 READ AES DATA  AES_FILENAME=["6106000.123"],
18                IELEM=[123], START_DATE=[0], END_DATE=[-364]
19 *%-----|-----|
20 COMPUTE API    APII=[50], APIK=[0.90]/day
21 *#*****
22 *# Pre Development Condition - Using NASHHYD and CN
23 *#*****
24 CONTINUOUS NASHYD  NHYD=["EastPre"], DT=[15]min, AREA=[39.35](ha),
25                DWF=[0](cms), CN/C=[65], IA=[5.5](mm),
26                N=[3], TP=[1.47]hrs,
27                Continuous simulation parameters:
28                IaREcper=[6](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
29                SK=[0.025]/(mm), InterEventTime=[ 12 ](hrs)
30                Baseflow simulation parameters:
31                BaseFlowOption=[1] , InitGWResVol=[ 0.0 ](mm), GWResK=[ 0.935
32                ](mm/day/mm)
33                VHydCond=[ 0.07 ](mm/hr), END=-1
34 *%-----|-----|
35 CONTINUOUS NASHYD  NHYD=["WestPre"], DT=[15]min, AREA=[39.85](ha),
36                DWF=[0](cms), CN/C=[61], IA=[5.5](mm),
37                N=[3], TP=[1.22]hrs,
38                Continuous simulation parameters:
39                IaREcper=[6](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
40                SK=[0.025]/(mm), InterEventTime=[ 12 ](hrs)
41                Baseflow simulation parameters:
42                BaseFlowOption=[1] , InitGWResVol=[ 0.0 ](mm), GWResK=[ 0.935
43                ](mm/day/mm)
44                VHydCond=[ 0.07 ](mm/hr), END=-1
45 *%-----|-----|
46 ADD HYD        NHYDsum=["Pre"], NHYDs to add=["WestPre"+"EastPre"]
47 *#*****
48 *# Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
49 *#*****
50 CONTINUOUS NASHYD  NHYD=["InfEastPre"], DT=[15]min, AREA=[39.35](ha),
51                DWF=[0](cms), CN/C=[99.99], IA=[5.5](mm),
52                N=[3], TP=[1.47]hrs,
53                Continuous simulation parameters:
54                IaREcper=[6](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
55                SK=[0.025]/(mm), InterEventTime=[ 12 ](hrs)
56                Baseflow simulation parameters:
57                BaseFlowOption=[1] , InitGWResVol=[ 0.0 ](mm), GWResK=[ 0.935
58                ](mm/day/mm)
59                VHydCond=[ 0.07 ](mm/hr), END=-1
60 *%-----|-----|
61 CONTINUOUS NASHYD  NHYD=["InfWestPre"], DT=[15]min, AREA=[39.85](ha),
62                DWF=[0](cms), CN/C=[99.99], IA=[5.5](mm),
63                N=[3], TP=[1.22]hrs,
64                Continuous simulation parameters:
65                IaREcper=[6](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
66                SK=[0.025]/(mm), InterEventTime=[ 12 ](hrs)
67                Baseflow simulation parameters:
68                BaseFlowOption=[1] , InitGWResVol=[ 0.0 ](mm), GWResK=[ 0.935
69                ](mm/day/mm)

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62          VHydCond=[ 0.07 ](mm/hr), END=-1
63 *%-----|-----|
64 ADD HYD          NHYDsum=["InfPre"], NHYDs to add=["InfWestPre"+"InfEastPre"]
65 *%-----|-----|
66 *#####|#####|
67 *# STORMS
68 *#####|#####|
69 START          TZERO=[1968.0101], METOUT=[2], NSTORM=[0], NRUN=[68]
70 *%-----|-----|
71 START          TZERO=[1969.0101], METOUT=[2], NSTORM=[0], NRUN=[69]
72 *%-----|-----|
73 START          TZERO=[1970.0101], METOUT=[2], NSTORM=[0], NRUN=[70]
74 *%-----|-----|
75 START          TZERO=[1971.0101], METOUT=[2], NSTORM=[0], NRUN=[71]
76 *%-----|-----|
77 START          TZERO=[1972.0101], METOUT=[2], NSTORM=[0], NRUN=[72]
78 *%-----|-----|
79 START          TZERO=[1973.0101], METOUT=[2], NSTORM=[0], NRUN=[73]
80 *%-----|-----|
81 START          TZERO=[1974.0101], METOUT=[2], NSTORM=[0], NRUN=[74]
82 *%-----|-----|
83 START          TZERO=[1975.0101], METOUT=[2], NSTORM=[0], NRUN=[75]
84 *%-----|-----|
85 START          TZERO=[1976.0101], METOUT=[2], NSTORM=[0], NRUN=[76]
86 *%-----|-----|
87 START          TZERO=[1977.0101], METOUT=[2], NSTORM=[0], NRUN=[77]
88 *%-----|-----|
89 START          TZERO=[1978.0101], METOUT=[2], NSTORM=[0], NRUN=[78]
90 *%-----|-----|
91 START          TZERO=[1979.0101], METOUT=[2], NSTORM=[0], NRUN=[79]
92 *%-----|-----|
93 START          TZERO=[1980.0101], METOUT=[2], NSTORM=[0], NRUN=[80]
94 *%-----|-----|
95 START          TZERO=[1981.0101], METOUT=[2], NSTORM=[0], NRUN=[81]
96 *%-----|-----|
97 START          TZERO=[1982.0101], METOUT=[2], NSTORM=[0], NRUN=[82]
98 *%-----|-----|
99 START          TZERO=[1983.0101], METOUT=[2], NSTORM=[0], NRUN=[83]
100 *%-----|-----|
101 START          TZERO=[1984.0101], METOUT=[2], NSTORM=[0], NRUN=[84]
102 *%-----|-----|
103 START          TZERO=[1985.0101], METOUT=[2], NSTORM=[0], NRUN=[85]
104 *%-----|-----|
105 START          TZERO=[1986.0101], METOUT=[2], NSTORM=[0], NRUN=[86]
106 *%-----|-----|
107 START          TZERO=[1987.0101], METOUT=[2], NSTORM=[0], NRUN=[87]
108 *%-----|-----|
109 START          TZERO=[1988.0101], METOUT=[2], NSTORM=[0], NRUN=[88]
110 *%-----|-----|
111 START          TZERO=[1989.0101], METOUT=[2], NSTORM=[0], NRUN=[89]
112 *%-----|-----|
113 START          TZERO=[1990.0101], METOUT=[2], NSTORM=[0], NRUN=[90]
114 *%-----|-----|
115 START          TZERO=[1991.0101], METOUT=[2], NSTORM=[0], NRUN=[91]
116 *%-----|-----|
117 START          TZERO=[1992.0101], METOUT=[2], NSTORM=[0], NRUN=[92]
118 *%-----|-----|
119 START          TZERO=[1993.0101], METOUT=[2], NSTORM=[0], NRUN=[93]
120 *%-----|-----|
121 START          TZERO=[1994.0101], METOUT=[2], NSTORM=[0], NRUN=[94]
122 *%-----|-----|
123 START          TZERO=[1995.0101], METOUT=[2], NSTORM=[0], NRUN=[95]
124 *%-----|-----|
125 START          TZERO=[1996.0101], METOUT=[2], NSTORM=[0], NRUN=[96]
126 *%-----|-----|
127 START          TZERO=[1997.0101], METOUT=[2], NSTORM=[0], NRUN=[97]
128 *%-----|-----|
129 START          TZERO=[1998.0101], METOUT=[2], NSTORM=[0], NRUN=[98]
130 *%-----|-----|

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131  START                TZERO=[1999.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[99]
132  *%-----|-----|
133  START                TZERO=[2000.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[100]
134  *%-----|-----|
135  *% MISSING FROM AES RAINFALL DATA
136  *%START              TZERO=[2001.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[101]
137  *%-----|-----|
138  START                TZERO=[2002.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[102]
139  *%-----|-----|
140  START                TZERO=[2003.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[103]
141  *%-----|-----|
142  FINISH
```


00361 # Pre Development Condition - Using NASHVD and CN
00362 *****
00363 RW071C00004-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00364 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .295 1970.0926.22:15 89.80 161 .000
00365 [CM: 61.0; W: 3.00; Tpe: 1.47]
00366 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00367 [InterEventTime= 12.00]
00368 RW071C00005-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00369 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 .311 1970.0926.22:00 88.22 158 .000
00370 [CM: 61.0; W: 3.00; Tpe: 1.47]
00371 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00372 [InterEventTime= 12.00]
00373 RW071C00006-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00374 ADD HYD + 15.0 02:InfEstPre 39.85 .311 1970.0926.22:00 88.22 n/a .000
00375 + 15.0 02:InfEstPre 39.35 .295 1970.0926.22:15 89.80 n/a .000
00376 SIM= 15.0 01:InfPre 79.20 .60 1970.0926.22:00 89.00 n/a .000
00377 [InterEventTime= 12.00]
00378 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
00379 *****
00380 RW071C00007-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00381 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.108 1970.0926.22:00 200.34 358 .000
00382 [CM:100.0; W: 3.00; Tpe: 1.47]
00383 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00384 [InterEventTime= 12.00]
00385 RW071C00008-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00386 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 1.329 1970.0926.21:45 200.34 358 .000
00387 [CM:100.0; W: 3.00; Tpe: 1.22]
00388 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00389 [InterEventTime= 12.00]
00390 RW071C00009-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00391 ADD HYD + 15.0 02:InfEstPre 39.85 1.329 1970.0926.21:45 200.34 n/a .000
00392 + 15.0 02:InfEstPre 39.35 1.108 1970.0926.22:00 200.34 n/a .000
00393 SIM= 15.0 01:InfPre 79.20 2.409 1970.0926.22:00 200.34 n/a .000
00394 *****
00395 # STORMS
00396 [METOUT= 2]
00397 [METOUT= 2 (Imperial, 2-metric output)]
00398 [NRUN = 071]
00399 [NRUN = 0073]
00400 *****
00401 # SWSHYMO Ver:5.02/Jan 2001 -CBETA / INPUT DATA FILE
00402 *****
00403 # Project Name : [Caivan Scitville West properties]
00404 # Project Number: [2267]
00405 # Date : [2021/12/14]
00406 # Modeler : [JB]
00407 # Company : [J.F. Sabourin and Associates]
00408 # License # : [2549237]
00409 # *****
00410 # Ottawa International Airport (1967 - 2003)
00411 *****
00412 # READ AES DATA
00413 [Filename = 610600.123]
00414 [Start_date= 1973.0101; End_date= 1973.1231]
00415 [DT= 60,min; Length= 8760,hrs; WetRes= 489; DryRes= 8211; PTO= 744.90]
00416 Maximum average rainfall intensities over
00417 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00418 20.60 30.80 31.10 31.10 29.80 39.00 39.00 39.00 39.00 mm/hr
00419 19730611 19730808 19730808 19730808 19730616 19730616 19730616 19730616 19730616
00420 Number of rainfall events per following interevent time
00421 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00422 200 102 66 20 4 0 0 0 0
00423 Number of events with at least the following durations
00424 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00425 200 102 66 20 4 0 0 0 0
00426 RW071C00010-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00427 COMPUTE API
00428 [APIIn= 50.00; APIkty= 9000; APIkdc= .9956]
00429 [APIAve= 62.22; APIave= 14.84; APIIne= .36]
00430 *****
00431 # Pre Development Condition - Using NASHVD and CN
00432 *****
00433 RW071C00011-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00434 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .216 1971.0810.16:45 68.80 132 .000
00435 [CM: 61.0; W: 3.00; Tpe: 1.47]
00436 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00437 [InterEventTime= 12.00]
00438 RW071C00012-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00439 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .220 1971.0810.16:30 67.79 130 .000
00440 [CM: 61.0; W: 3.00; Tpe: 1.22]
00441 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00442 [InterEventTime= 12.00]
00443 RW071C00013-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00444 ADD HYD + 15.0 02:EstPre 39.85 .220 1971.0810.16:30 67.79 n/a .000
00445 + 15.0 02:EstPre 39.35 .216 1971.0810.16:45 68.80 n/a .000
00446 SIM= 15.0 01:InfPre 79.20 .433 1971.0810.16:30 68.29 n/a .000
00447 *****
00448 # Pre Development Condition - Using NASHVD and CN
00449 *****
00450 RW071C00014-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00451 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .216 1971.0810.16:45 68.80 132 .000
00452 [CM: 61.0; W: 3.00; Tpe: 1.47]
00453 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00454 [InterEventTime= 12.00]
00455 RW071C00015-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00456 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .220 1971.0810.16:30 67.79 130 .000
00457 [CM: 61.0; W: 3.00; Tpe: 1.22]
00458 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00459 [InterEventTime= 12.00]
00460 RW071C00016-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00461 ADD HYD + 15.0 02:EstPre 39.85 .220 1971.0810.16:30 67.79 n/a .000
00462 + 15.0 02:EstPre 39.35 .216 1971.0810.16:45 68.80 n/a .000
00463 SIM= 15.0 01:InfPre 79.20 .433 1971.0810.16:30 68.29 n/a .000
00464 *****
00465 # Pre Development Condition - Using NASHVD and CN
00466 *****
00467 RW071C00017-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00468 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .216 1971.0810.16:45 68.80 132 .000
00469 [CM:100.0; W: 3.00; Tpe: 1.47]
00470 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00471 [InterEventTime= 12.00]
00472 RW071C00018-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00473 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .220 1971.0810.16:30 67.79 130 .000
00474 [CM:100.0; W: 3.00; Tpe: 1.22]
00475 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00476 [InterEventTime= 12.00]
00477 RW071C00019-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00478 ADD HYD + 15.0 02:EstPre 39.85 .220 1971.0810.16:30 67.79 n/a .000
00479 + 15.0 02:EstPre 39.35 .216 1971.0810.16:45 68.80 n/a .000
00480 SIM= 15.0 01:InfPre 79.20 .433 1971.0810.16:30 68.29 n/a .000
00481 *****
00482 # STORMS
00483 [METOUT= 2]
00484 [METOUT= 2 (Imperial, 2-metric output)]
00485 [NRUN = 071]
00486 [NRUN = 0073]
00487 *****
00488 # SWSHYMO Ver:5.02/Jan 2001 -CBETA / INPUT DATA FILE
00489 *****
00490 # Project Name : [Caivan Scitville West properties]
00491 # Project Number: [2267]
00492 # Date : [2021/12/14]
00493 # Modeler : [JB]
00494 # Company : [J.F. Sabourin and Associates]
00495 # License # : [2549237]
00496 # *****
00497 # Ottawa International Airport (1967 - 2003)
00498 *****
00499 # READ AES DATA
00500 [Filename = 610600.123]
00501 [Start_date= 1974.0101; End_date= 1974.1231]
00502 [DT= 60,min; Length= 8760,hrs; WetRes= 489; DryRes= 8211; PTO= 784.30]
00503 Maximum average rainfall intensities over
00504 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00505 20.60 30.80 31.10 31.10 29.80 39.00 39.00 39.00 39.00 mm/hr
00506 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718
00507 Number of rainfall events per following interevent time
00508 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00509 129 105 93 77 63 50 38 33 23
00510 Number of events with at least the following durations
00511 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00512 128 66 32 10 3 0 0 0 0
00513 RW071C00020-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00514 COMPUTE API
00515 [APIIn= 50.00; APIkty= 9000; APIkdc= .9956]
00516 [APIAve= 62.22; APIave= 14.84; APIIne= .36]
00517 *****
00518 # Pre Development Condition - Using NASHVD and CN
00519 *****
00520 RW071C00021-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00521 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .438 1972.0808.0:00 191.36 244 .000
00522 [CM: 61.0; W: 3.00; Tpe: 1.47]
00523 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00524 [InterEventTime= 12.00]
00525 RW071C00022-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00526 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .465 1972.0807.3:45 188.98 241 .000
00527 [CM: 61.0; W: 3.00; Tpe: 1.22]
00528 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00529 [InterEventTime= 12.00]
00530 RW071C00023-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00531 ADD HYD + 15.0 02:EstPre 39.35 .438 1972.0808.0:00 191.36 244 .000
00532 + 15.0 02:EstPre 39.85 .465 1972.0807.3:45 188.98 241 .000
00533 SIM= 15.0 01:InfPre 79.20 .855 1972.0808.0:00 191.36 244 .000
00534 *****
00535 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
00536 *****
00537 RW071C00024-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00538 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .465 1972.0807.3:45 188.98 241 .000
00539 [CM:100.0; W: 3.00; Tpe: 1.47]
00540 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00541 [InterEventTime= 12.00]
00542 RW071C00025-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00543 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .492 1972.0808.0:00 189.98 239 .000
00544 [CM: 61.0; W: 3.00; Tpe: 1.22]
00545 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00546 [InterEventTime= 12.00]
00547 RW071C00026-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00548 ADD HYD + 15.0 02:EstPre 39.35 .465 1972.0807.3:45 188.98 241 .000
00549 + 15.0 02:EstPre 39.85 .492 1972.0808.0:00 189.98 239 .000
00550 SIM= 15.0 01:InfPre 79.20 .912 1972.0808.0:00 189.98 239 .000
00551 *****
00552 # STORMS
00553 [METOUT= 2]
00554 [METOUT= 2 (Imperial, 2-metric output)]
00555 [NRUN = 073]
00556 [NRUN = 0073]
00557 *****
00558 # SWSHYMO Ver:5.02/Jan 2001 -CBETA / INPUT DATA FILE
00559 *****
00560 # Project Name : [Caivan Scitville West properties]
00561 # Project Number: [2267]
00562 # Date : [2021/12/14]
00563 # Modeler : [JB]
00564 # Company : [J.F. Sabourin and Associates]
00565 # License # : [2549237]
00566 # *****
00567 # Ottawa International Airport (1967 - 2003)
00568 *****
00569 # READ AES DATA
00570 [Filename = 610600.123]
00571 [Start_date= 1974.0101; End_date= 1974.1231]
00572 [DT= 60,min; Length= 8760,hrs; WetRes= 489; DryRes= 8440; PTO= 386.20]
00573 Maximum average rainfall intensities over
00574 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00575 20.60 30.80 31.10 31.10 29.80 39.00 39.00 39.00 39.00 mm/hr
00576 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718
00577 Number of rainfall events per following interevent time
00578 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00579 129 105 93 77 63 50 38 33 23
00580 Number of events with at least the following durations
00581 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00582 128 66 32 10 3 0 0 0 0
00583 RW071C00027-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00584 COMPUTE API
00585 [APIIn= 50.00; APIkty= 9000; APIkdc= .9956]
00586 [APIAve= 62.22; APIave= 14.84; APIIne= .36]
00587 *****
00588 # Pre Development Condition - Using NASHVD and CN
00589 *****
00590 RW071C00028-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00591 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .127 1974.0718.1:45 42.84 n/a .000
00592 [CM: 61.0; W: 3.00; Tpe: 1.47]
00593 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00594 [InterEventTime= 12.00]
00595 RW071C00029-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00596 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .150 1974.0718.1:45 42.84 n/a .000
00597 [CM: 61.0; W: 3.00; Tpe: 1.22]
00598 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00599 [InterEventTime= 12.00]
00600 RW071C00030-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00601 ADD HYD + 15.0 02:EstPre 39.35 .127 1974.0718.1:45 42.84 n/a .000
00602 + 15.0 02:EstPre 39.85 .150 1974.0718.1:45 42.84 n/a .000
00603 SIM= 15.0 01:InfPre 79.20 .254 1974.0718.1:45 42.84 n/a .000
00604 *****
00605 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
00606 *****
00607 RW071C00031-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00608 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .790 1974.0718.1:30 105.93 272 .000
00609 [CM:100.0; W: 3.00; Tpe: 1.47]
00610 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00611 [InterEventTime= 12.00]
00612 RW071C00032-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00613 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .917 1974.0718.1:30 105.93 272 .000
00614 [CM: 61.0; W: 3.00; Tpe: 1.22]
00615 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00616 [InterEventTime= 12.00]
00617 RW071C00033-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00618 ADD HYD + 15.0 02:EstPre 39.35 .790 1974.0718.1:30 105.93 272 .000
00619 + 15.0 02:EstPre 39.85 .917 1974.0718.1:30 105.93 272 .000
00620 SIM= 15.0 01:InfPre 79.20 .157 1974.0718.1:30 105.93 272 .000
00621 *****
00622 # STORMS
00623 [METOUT= 2]
00624 [METOUT= 2 (Imperial, 2-metric output)]
00625 [NRUN = 073]
00626 [NRUN = 0073]
00627 *****
00628 # SWSHYMO Ver:5.02/Jan 2001 -CBETA / INPUT DATA FILE
00629 *****
00630 # Project Name : [Caivan Scitville West properties]
00631 # Project Number: [2267]
00632 # Date : [2021/12/14]
00633 # Modeler : [JB]
00634 # Company : [J.F. Sabourin and Associates]
00635 # License # : [2549237]
00636 # *****
00637 # Ottawa International Airport (1967 - 2003)
00638 *****
00639 # READ AES DATA
00640 [Filename = 610600.123]
00641 [Start_date= 1974.0101; End_date= 1974.1231]
00642 [DT= 60,min; Length= 8760,hrs; WetRes= 489; DryRes= 8440; PTO= 386.20]
00643 Maximum average rainfall intensities over
00644 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00645 20.60 30.80 31.10 31.10 29.80 39.00 39.00 39.00 39.00 mm/hr
00646 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718
00647 Number of rainfall events per following interevent time
00648 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00649 129 105 93 77 63 50 38 33 23
00650 Number of events with at least the following durations
00651 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00652 128 66 32 10 3 0 0 0 0
00653 RW071C00034-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00654 COMPUTE API
00655 [APIIn= 50.00; APIkty= 9000; APIkdc= .9956]
00656 [APIAve= 62.22; APIave= 14.84; APIIne= .36]
00657 *****
00658 # Pre Development Condition - Using NASHVD and CN
00659 *****
00660 RW071C00035-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00661 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .127 1974.0718.1:45 42.84 n/a .000
00662 [CM: 61.0; W: 3.00; Tpe: 1.47]
00663 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00664 [InterEventTime= 12.00]
00665 RW071C00036-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00666 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .150 1974.0718.1:45 42.84 n/a .000
00667 [CM: 61.0; W: 3.00; Tpe: 1.22]
00668 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00669 [InterEventTime= 12.00]
00670 RW071C00037-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00671 ADD HYD + 15.0 02:EstPre 39.35 .127 1974.0718.1:45 42.84 n/a .000
00672 + 15.0 02:EstPre 39.85 .150 1974.0718.1:45 42.84 n/a .000
00673 SIM= 15.0 01:InfPre 79.20 .254 1974.0718.1:45 42.84 n/a .000
00674 *****
00675 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
00676 *****
00677 RW071C00038-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00678 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .790 1974.0718.1:30 105.93 272 .000
00679 [CM:100.0; W: 3.00; Tpe: 1.47]
00680 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00681 [InterEventTime= 12.00]
00682 RW071C00039-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00683 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .917 1974.0718.1:30 105.93 272 .000
00684 [CM: 61.0; W: 3.00; Tpe: 1.22]
00685 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00686 [InterEventTime= 12.00]
00687 RW071C00040-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00688 ADD HYD + 15.0 02:EstPre 39.35 .790 1974.0718.1:30 105.93 272 .000
00689 + 15.0 02:EstPre 39.85 .917 1974.0718.1:30 105.93 272 .000
00690 SIM= 15.0 01:InfPre 79.20 .157 1974.0718.1:30 105.93 272 .000
00691 *****
00692 # STORMS
00693 [METOUT= 2]
00694 [METOUT= 2 (Imperial, 2-metric output)]
00695 [NRUN = 073]
00696 [NRUN = 0073]
00697 *****
00698 # SWSHYMO Ver:5.02/Jan 2001 -CBETA / INPUT DATA FILE
00699 *****
00700 # Project Name : [Caivan Scitville West properties]
00701 # Project Number: [2267]
00702 # Date : [2021/12/14]
00703 # Modeler : [JB]
00704 # Company : [J.F. Sabourin and Associates]
00705 # License # : [2549237]
00706 # *****
00707 # Ottawa International Airport (1967 - 2003)
00708 *****
00709 # READ AES DATA
00710 [Filename = 610600.123]
00711 [Start_date= 1974.0101; End_date= 1974.1231]
00712 [DT= 60,min; Length= 8760,hrs; WetRes= 489; DryRes= 8440; PTO= 386.20]
00713 Maximum average rainfall intensities over
00714 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00715 20.60 30.80 31.10 31.10 29.80 39.00 39.00 39.00 39.00 mm/hr
00716 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718 19740718
00717 Number of rainfall events per following interevent time
00718 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00719 129 105 93 77 63 50 38 33 23
00720 Number of events with at least the following durations
00721 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00722 128 66 32 10 3 0 0 0 0
00723 RW071C00041-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00724 COMPUTE API
00725 [APIIn= 50.00; APIkty= 9000; APIkdc= .9956]
00726 [APIAve= 62.22; APIave= 14.84; APIIne= .36]
00727 *****
00728 # Pre Development Condition - Using NASHVD and CN
00729 *****
00730 RW071C00042-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00731 CONTINUOUS NASHVD 15.0 01:EstPre 39.35 .127 1974.0718.1:45 42.84 n/a .000
00732 [CM: 61.0; W: 3.00; Tpe: 1.47]
00733 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
00734 [InterEventTime= 12.00]
00735 RW071C00043-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DWfcm
00736 CONTINUOUS NASHVD 15.0 01:EstPre 39.85 .150 1974.0718.1:45 42.84 n/a .000
00737 [CM: 61.0; W: 3.00; Tpe: 1.22]
00738 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
00739 [InterEventTime= 12.00]
00740 RW071C00044-----DtmIn-ID:INVD-----AREAAh-GFEARcms-TpeakDate_hh:mm-----RvM-R.C-----DW

00721 R0074C00006 -----DtmIn-ID:INHYD-----AREAA-PEAKS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
00722 CONTINUOUS NASHVD 15.0 01:InfWestPre 39.85 .916 1974.0719, 1115 103.03 272 .000
00723 [CN=100.0; N= 3.00; Tm= 1.22]
00724 [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
00725 IntereventTime= 12.00


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01081 [METOUT= 2 (1=Imperial, 2=metric output)]
01082 [NFORM= 0 ]
01083 [NRUN = 0079 ]
01084 *****
01085 # SWMHYD0 Ver1.02/Jan 2001 <BETA> / INPUT DATA FILE
01086 *****
01087 # Project Name : [Calvan Strittville West properties]
01088 # Project Number: [2267]
01089 # Date : [2021/12/14]
01090 # Modeler : [JB]
01091 # Company : [J.F. Sabourin and Associates]
01092 # License # : [2549237]
01093 *****
01094 # Ottawa International Airport (1967 - 2003)
01095 R0079:CO002-----
01096 # READ AER DATA
01097 [Filename = 610600.123 ]
01098 [Start_date= 1979.0101; End_date= 1979.1231 ]
01099 [DT= 60.min; Length= 8760.hrs; WetHrs= 8214; PTO= 866.50]
01100 *****
01101 Maximum average rainfall intensities over
01102 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01103 34.90 22.00 14.67 7.33 5.14 2.63 1.75 1.31 .88 mm/hr
01104 34.90 44.00 44.00 44.00 61.70 63.00 63.00 63.00 63.00
01105 1979016 1979016 1979016 1979016 1979016 1979016 1979016 1979016 1979016 date
01106 *****
01107 Number of rainfall events per following interval time
01108 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01109 205 160 140 114 92 61 52 43 35
01110 *****
01111 Number of events with at least the following durations
01112 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01113 204 98 68 23 4 0 0 0 0
01114 *****
01115 COMPUTE API
01116 [APIIn= 50.00; APIKdy= 9000; APIKdt= .9956]
01117 [APIInx= 78.43; APIFav= 23.11; APIFmin= .13]
01118 *****
01119 # Pre Development Condition - Using NASHVD and CN
01120 *****
01121 CONTINUOUS NASHVD 15.0 01:InfEstPre AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01122 [CN= 65.0; N= 3.00; Tp= 1.47]
01123 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01124 [InterEventTime= 12.00]
01125 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 .406 1979.0616.15:00 198.19 n/a .000
01126 [CN= 61.0; N= 3.00; Tp= 1.22]
01127 [IARC= 6.00; SMIN= 43.01; SMAX=430.01; SK= .025]
01128 [InterEventTime= 12.00]
01129 R0079:CO006-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01130 ADD HYD + 15.0 02:InfEstPre 39.35 .382 1979.0616.15:00 198.36 n/a .000
01131 [CN= 61.0; N= 3.00; Tp= 1.22]
01132 [IARC= 6.00; SMIN= 43.01; SMAX=430.01; SK= .025]
01133 [InterEventTime= 12.00]
01134 *****
01135 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01136 *****
01137 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .382 1979.0616.15:00 198.36 n/a .000
01138 [CN=100.0; N= 3.00; Tp= 1.47]
01139 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01140 *****
01141 R0079:CO008-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01142 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 .153 1979.0616.14:45 400.19 n/a .000
01143 [CN=100.0; N= 3.00; Tp= 1.22]
01144 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01145 *****
01146 R0079:CO009-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01147 ADD HYD + 15.0 02:InfEstPre 39.35 .382 1979.0616.15:00 198.36 n/a .000
01148 [CN= 61.0; N= 3.00; Tp= 1.22]
01149 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01150 *****
01151 # STORMS
01152 *****
01153 ** END OF RUN : 79
01154 *****
01155 *****
01156 *****
01157 *****
01158 *****
01159 *****
01160 *****
01161 RUN:COMMAND#
01162 R0080:CO001-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01163 START [TZERO = .00 hrs on 19800101]
01164 [METOUT= 2 (1=Imperial, 2=metric output)]
01165 [NFORM= 0 ]
01166 [NRUN = 0082 ]
01167 *****
01168 # SWMHYD0 Ver1.02/Jan 2001 <BETA> / INPUT DATA FILE
01169 *****
01170 # Project Name : [Calvan Strittville West properties]
01171 # Project Number: [2267]
01172 # Date : [2021/12/14]
01173 # Modeler : [JB]
01174 # Company : [J.F. Sabourin and Associates]
01175 # License # : [2549237]
01176 *****
01177 # Ottawa International Airport (1967 - 2003)
01178 R0080:CO002-----
01179 # READ AER DATA
01180 [Filename= 610600.123 ]
01181 [Start_date= 1980.0101; End_date= 1980.1230]
01182 [DT= 60.min; Length= 8760.hrs; WetHrs= 8331; PTO= 822.00]
01183 *****
01184 Maximum average rainfall intensities over
01185 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01186 15.00 10.00 6.20 3.23 2.35 1.01 .84 .84 mm/hr
01187 15.00 18.40 19.50 28.30 38.80 43.80 46.60 48.60 62.00 mm
01188 1980080 1980080 1980025 1980025 1980031 1980032 1980026 1980027 1980002 date
01189 *****
01190 Number of rainfall events per following interval time
01191 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01192 175 141 129 91 64 47 42 25
01193 *****
01194 Number of events with at least the following durations
01195 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01196 174 88 50 13 3 0 0 0 0
01197 *****
01198 COMPUTE API
01199 [APIIn= 50.00; APIKdy= 9000; APIKdt= .9956]
01200 [APIInx= 88.74; APIFav= 17.50; APIFmin= .06]
01201 *****
01202 # Pre Development Condition - Using NASHVD and CN
01203 *****
01204 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .413 1980.0901.21:30 88.71 143 .000
01205 [CN= 65.0; N= 3.00; Tp= 1.47]
01206 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01207 [InterEventTime= 12.00]
01208 R0080:CO005-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01209 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 .115 1980.0901.21:15 86.82 140 .000
01210 [CN= 61.0; N= 3.00; Tp= 1.22]
01211 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
01212 [InterEventTime= 12.00]
01213 R0080:CO006-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01214 ADD HYD + 15.0 02:InfEstPre 39.85 .115 1980.0901.21:15 86.82 n/a .000
01215 [CN= 61.0; N= 3.00; Tp= 1.22]
01216 [IARC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
01217 [InterEventTime= 12.00]
01218 *****
01219 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01220 *****
01221 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .454 1980.1025.18:15 210.11 338 .000
01222 [CN=100.0; N= 3.00; Tp= 1.47]
01223 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01224 [InterEventTime= 12.00]
01225 R0080:CO008-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01226 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 .494 1980.1025.17:45 210.11 338 .000
01227 [CN=100.0; N= 3.00; Tp= 1.22]
01228 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01229 [InterEventTime= 12.00]
01230 R0080:CO009-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01231 ADD HYD + 15.0 02:InfEstPre 39.85 .494 1980.1025.17:45 210.11 n/a .000
01232 [CN= 61.0; N= 3.00; Tp= 1.22]
01233 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01234 *****
01235 # STORMS
01236 *****
01237 ** END OF RUN : 80
01238 *****
01239 *****
01240 *****
01241 *****
01242 *****
01243 *****
01244 *****
01245 RUN:COMMAND#
01246 R0081:CO001-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01247 START [TZERO = .00 hrs on 19801010]
01248 [METOUT= 2 (1=Imperial, 2=metric output)]
01249 [NFORM= 0 ]
01250 [NRUN = 0081 ]
01251 *****
01252 # SWMHYD0 Ver1.02/Jan 2001 <BETA> / INPUT DATA FILE
01253 *****
01254 # Project Name : [Calvan Strittville West properties]
01255 # Project Number: [2267]
01256 # Date : [2021/12/14]
01257 # Modeler : [JB]
01258 # Company : [J.F. Sabourin and Associates]
01259 # License # : [2549237]
01260 *****
01261 # Ottawa International Airport (1967 - 2003)
01262 R0081:CO002-----
01263 # READ AER DATA
01264 [Filename = 610600.123 ]
01265 [Start_date= 1981.0101; End_date= 1981.1231 ]
01266 [DT= 60.min; Length= 8760.hrs; WetHrs= 426; DryHrs= 8239; PTO= 596.10]
01267 *****
01268 Maximum average rainfall intensities over
01269 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01270 35.30 21.85 25.23 15.70 8.03 4.58 3.22 2.41 1.61 mm/hr
01271 35.30 41.80 41.80 41.80 61.70 63.00 63.00 63.00 63.00
01272 1981001 1981001 1981001 1981001 1981001 1981001 1981001 1981001 1981001 date
01273 *****
01274 Number of rainfall events per following interval time
01275 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01276 242 188 156 122 93 70 57 45 27
01277 *****
01278 Number of events with at least the following durations
01279 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01280 241 129 79 29 4 0 0 0 0
01281 *****
01282 COMPUTE API
01283 [APIIn= 50.00; APIKdy= 9000; APIKdt= .9956]
01284 [APIInx= 116.15; APIFav= 25.69; APIFmin= .26]
01285 *****
01286 # Pre Development Condition - Using NASHVD and CN
01287 *****
01288 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.216 1981.0805.4:15 226.21 242 .000
01289 [CN= 65.0; N= 3.00; Tp= 1.47]
01290 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01291 [InterEventTime= 12.00]
01292 R0081:CO003-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01293 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 1.206 1981.0805.3:30 222.76 238 .000
01294 [CN= 61.0; N= 3.00; Tp= 1.22]
01295 [IARC= 6.00; SMIN= 44.01; SMAX=430.01; SK= .025]
01296 [InterEventTime= 12.00]
01297 R0081:CO004-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01298 ADD HYD + 15.0 02:InfEstPre 39.35 1.206 1981.0805.3:30 222.76 n/a .000
01299 [CN= 65.0; N= 3.00; Tp= 1.47]
01300 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01301 *****
01302 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01303 *****
01304 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.216 1981.0805.4:15 226.21 n/a .000
01305 [CN= 65.0; N= 3.00; Tp= 1.47]
01306 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01307 [InterEventTime= 12.00]
01308 R0081:CO007-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01309 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 1.206 1981.0805.3:30 222.76 n/a .000
01310 [CN= 61.0; N= 3.00; Tp= 1.22]
01311 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01312 [InterEventTime= 12.00]
01313 R0081:CO008-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01314 ADD HYD + 15.0 02:InfEstPre 39.35 2.274 1981.0805.3:30 399.82 n/a .000
01315 [CN= 61.0; N= 3.00; Tp= 1.22]
01316 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01317 *****
01318 # STORMS
01319 *****
01320 ** END OF RUN : 81
01321 *****
01322 *****
01323 *****
01324 *****
01325 *****
01326 *****
01327 *****
01328 *****
01329 *****
01330 *****
01331 *****
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01261 # Ottawa International Airport (1967 - 2003)
01262 R0082:CO002-----
01263 # READ AER DATA
01264 [Filename = 610600.123 ]
01265 [Start_date= 1982.0101; End_date= 1982.1231 ]
01266 [DT= 60.min; Length= 8760.hrs; WetHrs= 426; DryHrs= 8239; PTO= 596.10]
01267 *****
01268 Maximum average rainfall intensities over
01269 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01270 35.30 21.85 25.23 15.70 8.03 4.58 3.22 2.41 1.61 mm/hr
01271 35.30 41.80 41.80 41.80 61.70 63.00 63.00 63.00 63.00
01272 1982001 1982001 1982001 1982001 1982001 1982001 1982001 1982001 1982001 date
01273 *****
01274 Number of rainfall events per following interval time
01275 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01276 242 188 156 122 93 70 57 45 27
01277 *****
01278 Number of events with at least the following durations
01279 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01280 241 129 79 29 4 0 0 0 0
01281 *****
01282 COMPUTE API
01283 [APIIn= 50.00; APIKdy= 9000; APIKdt= .9956]
01284 [APIInx= 116.15; APIFav= 25.69; APIFmin= .26]
01285 *****
01286 # Pre Development Condition - Using NASHVD and CN
01287 *****
01288 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.216 1981.0805.4:15 226.21 242 .000
01289 [CN= 65.0; N= 3.00; Tp= 1.47]
01290 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01291 [InterEventTime= 12.00]
01292 R0081:CO003-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01293 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 1.206 1981.0805.3:30 222.76 238 .000
01294 [CN= 61.0; N= 3.00; Tp= 1.22]
01295 [IARC= 6.00; SMIN= 44.01; SMAX=430.01; SK= .025]
01296 [InterEventTime= 12.00]
01297 R0081:CO004-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01298 ADD HYD + 15.0 02:InfEstPre 39.35 1.206 1981.0805.3:30 222.76 n/a .000
01299 [CN= 65.0; N= 3.00; Tp= 1.47]
01300 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01301 *****
01302 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01303 *****
01304 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.216 1981.0805.4:15 226.21 n/a .000
01305 [CN= 65.0; N= 3.00; Tp= 1.47]
01306 [IARC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
01307 [InterEventTime= 12.00]
01308 R0081:CO007-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01309 CONTINUOUS NASHVD 15.0 01:InfEstPre 39.85 1.206 1981.0805.3:30 222.76 n/a .000
01310 [CN= 61.0; N= 3.00; Tp= 1.22]
01311 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01312 [InterEventTime= 12.00]
01313 R0081:CO008-----AREAA-QPEARms-TpeakDate_hh:mm--RvM-R.C.--DWfms
01314 ADD HYD + 15.0 02:InfEstPre 39.35 2.274 1981.0805.3:30 399.82 n/a .000
01315 [CN= 61.0; N= 3.00; Tp= 1.22]
01316 [IARC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
01317 *****
01318 # STORMS
01319 *****
01320 ** END OF RUN : 81
01321 *****
01322 *****
01323 *****
01324 *****
01325 *****
01326 *****
01327 *****
01328 *****
01329 *****
01330 *****
01331 *****
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01433 *****
01434 *****
01435 *****
01436 *****
01437 *****
01438 *****
01439 *****
01440 *****

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01801 R0887C00000-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01802 ADD HYD + 15.0 02:InfPrc 39.35 .298 1987.0724_15:45 90.24 n/a .000
01803 SUM = 15.0 02:InfPrc 39.35 .298 1987.0724_15:45 90.24 n/a .000
01804 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 .298 1987.0724_15:45 90.24 n/a .000
01805 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01806 * Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01807 *****
01808 R0887C00007-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01809 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 1.041 1987.0724_15:30 198.26 .310 .000
01810 [CN:100.0; N: 3.00; Tm: 1.47]
01811 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01812 *****
01813 R0887C00008-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01814 CONTINUOUS NASHVD 15.0 01:InfPrc 39.85 1.152 1987.0724_15:15 198.26 .310 .000
01815 [CN:100.0; N: 3.00; Tm: 1.22]
01816 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01817 *****
01818 R0887C00009-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01819 ADD HYD + 15.0 02:InfPrc 39.85 1.152 1987.0724_15:15 198.26 n/a .000
01820 SUM = 15.0 02:InfPrc 39.85 1.152 1987.0724_15:15 198.26 n/a .000
01821 CONTINUOUS NASHVD 15.0 01:InfPrc 79.20 2.181 1987.0724_15:30 198.26 n/a .000
01822 *****
01823 * STORMS
01824 *****
01825 ** END OF RUN : 87
01826 *****
01827 *****
01828 *****
01829 *****
01830 *****
01831 *****
01832 *****
01833 RUN:COMMAND#
01834 R0888C00001-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01835 START [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01836 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01837 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01838 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01839 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01840 *****
01841 * SWM3D Ver:5.02/Jan 2001 <SBTA> / INPUT DATA FILE
01842 *****
01843 # Project Name : [C:\rain\Stittville West properties]
01844 # Project Number : [2267]
01845 # Date : [2021/12/14]
01846 # Modeler : [JFB]
01847 # Company : [J.F. Sabourin and Associates]
01848 # License # : [254923]
01849 *****
01850 # Ottawa International Airport (1967 - 2003)
01851 *****
01852 R0889C00002-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01853 READ AES DATA [Filename = 6106000.123 ]
01854 [Start_date = 1988.01:01; End_date = 1988.12:31 ]
01855 [DT: 60 min; Length = 8760 hrs; WetHrs = 486; DryHrs = 8274; PTO: 643.20]
01856 *****
01857 Maximum average rainfall intensities over
01858 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs mm/hr
01859 25.50 36.40 44.20 45.40 45.40 45.40 45.40 45.40 45.40
01860 198807 198807 198807 198807 198807 198807 198807 198807 198807
01861 Number of rainfall events per following interval time
01862 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01863 183 141 121 85 68
01864 Number of events with at least the following durations 48 41 25
01865 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01866 182 101 70 19 3 0 0 0
01867 *****
01868 R0889C00003-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01869 COMPUTE API [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01870 [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01871 [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01872 *****
01873 # Pre Development Condition - Using NASHVD and CN
01874 *****
01875 R0889C00004-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01876 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 .366 1988.0625_14:00 106.79 .166 .000
01877 [CN:60.0; N: 3.00; Tm: 1.47]
01878 [IAREC 6.00; SMIN: 54.78; SMAX:365.23; SK: .025]
01879 *****
01880 R0889C00005-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01881 CONTINUOUS NASHVD 15.0 01:InfPrc 39.85 .383 1988.0625_13:45 104.72 .163 .000
01882 [CN:60.0; N: 3.00; Tm: 1.47]
01883 [IAREC 6.00; SMIN: 64.50; SMAX:430.01; SK: .025]
01884 *****
01885 R0889C00006-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01886 ADD HYD + 15.0 02:InfPrc 39.85 .383 1988.0625_13:45 104.72 n/a .000
01887 SUM = 15.0 02:InfPrc 39.85 .383 1988.0625_13:45 104.72 n/a .000
01888 CONTINUOUS NASHVD 15.0 01:InfPrc 79.20 .747 1988.0625_13:45 105.75 n/a .000
01889 *****
01890 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01891 *****
01892 R0889C00007-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01893 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 1.214 1988.0625_13:30 224.71 .349 .000
01894 [CN:100.0; N: 3.00; Tm: 1.47]
01895 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01896 *****
01897 R0889C00008-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01898 CONTINUOUS NASHVD 15.0 01:InfPrc 39.85 1.397 1988.0625_13:30 224.71 .349 .000
01899 [CN:100.0; N: 3.00; Tm: 1.22]
01900 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01901 *****
01902 R0889C00009-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01903 ADD HYD + 15.0 02:InfPrc 39.85 1.397 1988.0625_13:30 224.71 n/a .000
01904 SUM = 15.0 02:InfPrc 39.85 1.397 1988.0625_13:30 224.71 n/a .000
01905 CONTINUOUS NASHVD 15.0 01:InfPrc 79.20 2.611 1988.0625_13:30 224.71 n/a .000
01906 *****
01907 * STORMS
01908 *****
01909 ** END OF RUN : 88
01910 *****
01911 *****
01912 *****
01913 *****
01914 *****
01915 *****
01916 *****
01917 RUN:COMMAND#
01918 R0889C00010-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01919 START [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01920 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01921 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01922 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01923 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01924 *****
01925 * SWM3D Ver:5.02/Jan 2001 <SBTA> / INPUT DATA FILE
01926 *****
01927 # Project Name : [C:\rain\Stittville West properties]
01928 # Project Number : [2267]
01929 # Date : [2021/12/14]
01930 # Modeler : [JFB]
01931 # Company : [J.F. Sabourin and Associates]
01932 # License # : [254923]
01933 *****
01934 # Ottawa International Airport (1967 - 2003)
01935 *****
01936 R0889C00011-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01937 READ AES DATA [Filename = 6106000.123 ]
01938 [Start_date = 1989.01:01; End_date = 1989.12:31 ]
01939 [DT: 60 min; Length = 8640 hrs; WetHrs = 421; DryHrs = 7619; PTO: 522.50]
01940 *****
01941 Maximum average rainfall intensities over
01942 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs mm/hr
01943 22.70 32.60 39.80 5.75 3.03 1.67 1.14 .86 .50
01944 198907 198907 198907 198907 198907 198907 198907 198907 198907
01945 Number of rainfall events per following interval time
01946 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01947 170 137 120 101 71 52 40 36 29
01948 Number of events with at least the following durations 48 41 24 18 14 10 7 4 3
01949 169 81 49 17 4 0 0 0 0
01950 *****
01951 R0889C00012-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01952 COMPUTE API [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01953 [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01954 [APIIn: 50.00; APIKdy: 9000; APIKdt: .9956]
01955 *****
01956 # Pre Development Condition - Using NASHVD and CN
01957 *****
01958 R0889C00013-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01959 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 .144 1989.0727_16:30 66.36 .127 .000
01960 [IAREC 6.00; SMIN: 1.39; SMAX:365.23; SK: .025]
01961 *****
01962 R0889C00014-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01963 CONTINUOUS NASHVD 15.0 01:InfPrc 39.85 .145 1989.0727_16:00 64.85 .124 .000
01964 [IAREC 6.00; SMIN: 1.39; SMAX:430.01; SK: .025]
01965 *****
01966 R0889C00015-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01967 ADD HYD + 15.0 02:InfPrc 39.85 .145 1989.0727_16:00 64.85 n/a .000
01968 SUM = 15.0 02:InfPrc 39.85 .145 1989.0727_16:00 64.85 n/a .000
01969 CONTINUOUS NASHVD 15.0 01:InfPrc 79.20 2.181 1989.0727_16:15 165.82 .317 .000
01970 *****
01971 # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
01972 *****
01973 R0889C00016-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01974 CONTINUOUS NASHVD 15.0 01:InfPrc 39.35 .822 1989.0727_16:15 165.82 .317 .000
01975 [CN:100.0; N: 3.00; Tm: 1.47]
01976 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]
01977 *****
01978 R0889C00017-----DtmIn-ID:INHYD-----AREAh-QFEARcMs-TpeakDate_hh:mm-----RvM-R-C-----DMFcms
01979 CONTINUOUS NASHVD 15.0 01:InfPrc 39.85 1.152 1989.0727_15:15 198.26 .310 .000
01980 [CN:100.0; N: 3.00; Tm: 1.22]
01981 [IAREC 6.00; SMIN: 1.39; SMAX: 9.24; SK: .025]

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02161) ** END OF RUN : 91
02162)
02163)
02164)
02165)
02166)
02167)
02168)
02169) RUN# COMMAND#
02170) R0992.C0001#
02171) START
02172) [TZERO = .00 hrs on 19930101]
02173) [METOUT= 2 (1=Imperial, 2=metric output)]
02174) [NFORM= 0]
02175) [NRUN = 0092 ]
02176)
02177) # SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02178) # Project Name : [Caivan Strittville West properties]
02179) # Date : [2021/12/14]
02180) # Project Number: [2267]
02181) # Modeler : [JB]
02182) # Company : [J.F. Sabourin and Associates]
02183) # License # : [2549237]
02184)
02185) # Ottawa International Airport (1967 - 2003)
02186)
02187) # READ AES DATA
02188) R0992.C0002-----
02189) [Filename = 610600.123 ]
02190) [Start_date= 1994.0101; End_date= 1994.1231]
02191) [DTF= 60,min; Length= 8760,hrs; WetRcs= 550; DryRcs= 8210; PTOF= 730.20]
02192)
02193) Maximum average rainfall intensities over
02194) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02195) 31.50 18.00 13.90 7.22 4.14 2.56 1.51 1.01 1.02 mm/hr
02196) 31.50 36.00 39.90 43.30 49.70 54.20 54.20 72.60 73.60 mm
02197) 19930104 19930104 19930104 19930104 19930104 19930104 19930104 19930104 19930104 date
02198) Number of rainfall events per following interval time
02199) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02200) 1 1 1 1 1 1 1 1 1
02201) Number of events with at least the following durations
02202) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02203) 220 113 73 18 4 0 0 0 0
02204)
02205) COMPUTE API
02206) [APIInl= 50.00; APIkty= 9000; APIkdt= .9956]
02207) [APIkty= 50.00; APIkty= 9000; APIkdt= .9956]
02208)
02209) # Pre Development Condition - Using NASHVD and CN
02210)
02211) R0992.C0004-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02212) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .519 1992.0717,19.30 137.35 1188 .000
02213) [CN= 65.0; N= 3.00; Tpe= 1.47]
02214) [IAREC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02215) [InterEventTime= 12.00]
02216) R0992.C0005-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02217) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .528 1992.0717,19.30 135.31 1186 .000
02218) [CN= 61.0; N= 3.00; Tpe= 1.22]
02219) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02220) [InterEventTime= 12.00]
02221) R0992.C0006-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02222) ADD HYD + 15.0 02:InfEstPre 39.35 .528 1992.0717,19.30 135.31 n/a .000
02223) [CN= 61.0; N= 3.00; Tpe= 1.22]
02224) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02225) [InterEventTime= 12.00]
02226) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02227)
02228) R0992.C0007-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02229) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.280 1992.0717,19.15 273.86 1375 .000
02230) [CN= 10.0; N= 3.00; Tpe= 1.47]
02231) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02232) [InterEventTime= 12.00]
02233) R0992.C0008-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02234) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 1.453 1992.0804,14.45 273.86 1375 .000
02235) [CN= 10.0; N= 3.00; Tpe= 1.22]
02236) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02237) [InterEventTime= 12.00]
02238) R0992.C0009-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02239) ADD HYD + 15.0 02:InfEstPre 39.35 1.453 1992.0804,14.45 273.86 n/a .000
02240) [CN= 10.0; N= 3.00; Tpe= 1.22]
02241) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02242) [InterEventTime= 12.00]
02243) # STORMS
02244)
02245) ** END OF RUN : 91
02246)
02247)
02248)
02249)
02250)
02251)
02252)
02253) RUN# COMMAND#
02254) R0993.C0001#
02255) START
02256) [TZERO = .00 hrs on 19930101]
02257) [METOUT= 2 (1=Imperial, 2=metric output)]
02258) [NFORM= 0]
02259) [NRUN = 0093 ]
02260)
02261) # SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02262) # Project Name : [Caivan Strittville West properties]
02263) # Date : [2021/12/14]
02264) # Project Number: [2267]
02265) # Modeler : [JB]
02266) # Company : [J.F. Sabourin and Associates]
02267) # License # : [2549237]
02268)
02269) # Ottawa International Airport (1967 - 2003)
02270)
02271) # READ AES DATA
02272) R0993.C0002-----
02273) [Filename = 610600.123 ]
02274) [Start_date= 1993.0101; End_date= 1993.1231]
02275) [DTF= 60,min; Length= 8760,hrs; WetRcs= 584; DryRcs= 8176; PTOF= 721.10]
02276)
02277) Maximum average rainfall intensities over
02278) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02279) 12.60 6.60 4.83 3.72 3.58 2.31 1.61 1.21 .81 mm/hr
02280) 12.60 12.60 14.50 22.30 43.00 55.10 58.10 58.10 58.10 mm
02281) 19930703 19930703 19931127 19931128 19931128 19931128 19931128 19931128 19931128 date
02282)
02283) Number of rainfall events per following interval time
02284) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02285) 1 1 1 1 1 1 1 1 1
02286) Number of events with at least the following durations
02287) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02288) 189 110 66 27 7 2 0 0 0
02289)
02290) COMPUTE API
02291) [APIInl= 50.00; APIkty= 9000; APIkdt= .9956]
02292) [APIkty= 50.00; APIkty= 9000; APIkdt= .9956]
02293) [APIkty= 66.42; APIkty= 20.01; APIkdt= .11]
02294)
02295) # Pre Development Condition - Using NASHVD and CN
02296)
02297) R0993.C0004-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02298) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .194 1993.1128, 8.145 94.79 131 .000
02299) [CN= 65.0; N= 3.00; Tpe= 1.47]
02300) [IAREC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02301) [InterEventTime= 12.00]
02302) R0993.C0005-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02303) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .188 1993.1128, 8.130 92.42 128 .000
02304) [CN= 61.0; N= 3.00; Tpe= 1.22]
02305) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02306) [InterEventTime= 12.00]
02307) R0993.C0006-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02308) ADD HYD + 15.0 02:InfEstPre 39.35 .188 1993.1128, 8.130 92.42 n/a .000
02309) [CN= 61.0; N= 3.00; Tpe= 1.22]
02310) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02311) [InterEventTime= 12.00]
02312) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02313)
02314) R0993.C0007-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02315) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .458 1993.1128, 8.130 235.03 326 .000
02316) [CN= 10.0; N= 3.00; Tpe= 1.47]
02317) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02318) [InterEventTime= 12.00]
02319) R0993.C0008-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02320) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .485 1993.1128, 8.115 235.03 326 .000
02321) [CN= 10.0; N= 3.00; Tpe= 1.22]
02322) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02323) [InterEventTime= 12.00]
02324) ADD HYD + 15.0 02:InfEstPre 39.35 .485 1993.1128, 8.115 235.03 n/a .000
02325) [CN= 10.0; N= 3.00; Tpe= 1.22]
02326) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02327) [InterEventTime= 12.00]
02328) # STORMS
02329)
02330) ** END OF RUN : 93
02331)
02332)
02333)
02334)
02335)
02336)
02337) RUN# COMMAND#
02338) R0994.C0001#
02339) START
02340) [TZERO = .00 hrs on 19940101]
02341) [METOUT= 2 (1=Imperial, 2=metric output)]
02342) [NFORM= 0]
02343) [NRUN = 0094 ]
02344)
02345) # SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02346) # Project Name : [Caivan Strittville West properties]
02347) # Date : [2021/12/14]
02348) # Project Number: [2267]
02349) # Modeler : [JB]
02350) # Company : [J.F. Sabourin and Associates]
02351) # License # : [2549237]
02352)
02353) # Ottawa International Airport (1967 - 2003)
02354)
02355) # READ AES DATA
02356) R0994.C0002-----
02357) [Filename = 610600.123 ]
02358) [Start_date= 1994.0101; End_date= 1994.1231]
02359) [DTF= 60,min; Length= 8760,hrs; WetRcs= 328; DryRcs= 6248; PTOF= 527.00]
02360)
02361) Maximum average rainfall intensities over
02362) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02363) 22.60 11.90 8.43 5.42 2.92 1.79 1.19 .89 1.15 mm/hr
02364) 22.60 22.60 25.30 32.50 35.00 42.90 42.90 42.90 42.90 mm
02365) 19940229 19940229 19940229 19940229 19940229 19940229 19940229 19940229 19940229 date
02366) Number of rainfall events per following interval time
02367) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02368) 131 110 96 75 60 46 37 32 23
02369) Number of events with at least the following durations
02370) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02371) 130 70 46 12 1 0 0 0 0
02372)
02373) R0994.C0003-----
02374) COMPUTE API
02375) [APIInl= 50.00; APIkty= 9000; APIkdt= .9956]
02376) [APIkty= 50.00; APIkty= 9000; APIkdt= .9956]
02377) [APIkty= 97.84; APIkty= 19.15; APIkdt= .02]
02378)
02379) # Pre Development Condition - Using NASHVD and CN
02380)
02381) R0994.C0004-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02382) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .179 1994.0621,13.00 103.78 197 .000
02383) [CN= 65.0; N= 3.00; Tpe= 1.47]
02384) [IAREC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02385) [InterEventTime= 12.00]
02386) R0994.C0005-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02387) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .179 1994.0621,13.00 103.78 197 .000
02388) [CN= 61.0; N= 3.00; Tpe= 1.22]
02389) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02390) [InterEventTime= 12.00]
02391) R0994.C0006-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02392) ADD HYD + 15.0 02:InfEstPre 39.35 .179 1994.0621,13.00 103.78 n/a .000
02393) [CN= 100.0; N= 3.00; Tpe= 1.22]
02394) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02395) [InterEventTime= 12.00]
02396) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02397)
02398) R0994.C0007-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02399) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .696 1994.0621,12.15 204.63 388 .000
02400) [CN= 100.0; N= 3.00; Tpe= 1.47]
02401) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02402) [InterEventTime= 12.00]
02403) R0994.C0008-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02404) ADD HYD + 15.0 02:InfEstPre 39.35 .696 1994.0621,12.15 204.63 n/a .000
02405) [CN= 100.0; N= 3.00; Tpe= 1.22]
02406) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02407) [InterEventTime= 12.00]
02408) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02409)
02410) R0994.C0009-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02411) ADD HYD + 15.0 02:InfEstPre 39.35 .696 1994.0621,12.15 204.63 n/a .000
02412) [CN= 10.0; N= 3.00; Tpe= 1.47]
02413) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02414) [InterEventTime= 12.00]
02415) # STORMS
02416)
02417) ** END OF RUN : 94
02418)
02419)
02420)
02421) RUN# COMMAND#
02422) R0995.C0001#
02423) START
02424) [TZERO = .00 hrs on 19950101]
02425) [METOUT= 2 (1=Imperial, 2=metric output)]
02426) [NFORM= 0]
02427) [NRUN = 0095 ]
02428)
02429) # SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02430) # Project Name : [Caivan Strittville West properties]
02431) # Date : [2021/12/14]
02432) # Project Number: [2267]
02433) # Modeler : [JB]
02434) # Company : [J.F. Sabourin and Associates]
02435) # License # : [2549237]
02436)
02437) # Ottawa International Airport (1967 - 2003)
02438)
02439) # READ AES DATA
02440) R0995.C0002-----
02441) [Filename = 610600.123 ]
02442) [Start_date= 1995.0101; End_date= 1995.1231]
02443) [DTF= 60,min; Length= 5112,hrs; WetRcs= 228; DryRcs= 4884; PTOF= 321.60]
02444)
02445) Maximum average rainfall intensities over
02446) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02447) 16.90 8.90 6.30 3.31 2.21 1.65 1.10 1.10 mm/hr
02448) 16.90 26.50 38.80 46.20 75.80 79.40 79.40 79.40 mm
02449) 19950603 19950603 19950603 19950603 19950603 19950603 19950603 19950603 19950603 date
02450) Number of rainfall events per following interval time
02451) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02452) 66 27 23 10 9 1 0 0 0
02453) Number of events with at least the following durations
02454) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02455) 66 27 23 10 9 1 0 0 0
02456)
02457) R0995.C0003-----
02458) COMPUTE API
02459) [APIInl= 50.00; APIkty= 9000; APIkdt= .9956]
02460) [APIkty= 50.00; APIkty= 9000; APIkdt= .9956]
02461) [APIkty= 95.54; APIkty= 16.52; APIkdt= .49]
02462)
02463) # Pre Development Condition - Using NASHVD and CN
02464)
02465) R0995.C0004-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02466) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .640 1995.0603, 9.45 93.16 290 .000
02467) [CN= 61.0; N= 3.00; Tpe= 1.47]
02468) [IAREC= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02469) [InterEventTime= 12.00]
02470) R0995.C0005-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02471) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .660 1995.0603, 9.30 91.61 285 .000
02472) [CN= 61.0; N= 3.00; Tpe= 1.22]
02473) [IAREC= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02474) [InterEventTime= 12.00]
02475) R0995.C0006-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02476) ADD HYD + 15.0 02:InfEstPre 39.35 .660 1995.0603, 9.30 91.61 n/a .000
02477) [CN= 100.0; N= 3.00; Tpe= 1.22]
02478) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02479) [InterEventTime= 12.00]
02480) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02481)
02482) R0995.C0007-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02483) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .898 1995.0603, 9.30 158.70 493 .000
02484) [CN= 100.0; N= 3.00; Tpe= 1.47]
02485) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02486) [InterEventTime= 12.00]
02487) R0995.C0008-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02488) CONTINUOUS NASHVD 15.0 01:InfEstPre 39.35 .898 1995.0603, 9.15 158.70 493 .000
02489) [CN= 100.0; N= 3.00; Tpe= 1.22]
02490) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02491) [InterEventTime= 12.00]
02492) # Pre Development Condition - Using NASHVD and CN - No INFILTRATION
02493)
02494) R0995.C0009-----Dtain-ID:INVD-----AREAA-QFEARCS-TpeakDate_hh:mm-----RvM-R-C-----DMFCS
02495) ADD HYD + 15.0 02:InfEstPre 39.35 .898 1995.0603, 9.15 158.70 n/a .000
02496) [CN= 10.0; N= 3.00; Tpe= 1.47]
02497) [IAREC= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02498) [InterEventTime= 12.00]
02499) # STORMS
02500)
02501) ** END OF RUN : 95
02502)
02503)
02504)
02505)
02506) RUN# COMMAND#
02507) R0996.C0001#
02508) START
02509) [TZERO = .00 hrs on 19960101]
02510) [METOUT= 2 (1=Imperial, 2=metric output)]
02511) [NFORM= 0]
02512) [NRUN = 0096 ]
02513)
02514) # SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02515) # Project Name : [Caivan Strittville West properties]
02516) # Date : [2021/12/14]
02517) # Project Number: [2267]
02518) # Modeler : [JB]
02519) # Company : [J.F. Sabourin and Associates]
02520) # License # : [2549237]

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02521 *****
02522 # Ottawa International Airport (1967 - 2003)
02523 # READ AES DATA
02524 [Filename = 610600.123
02525 [Start_date = 1999.0101; End_date = 1999.1231]
02526 [DTF = 60; min: Length= 6552; hrs: WetHrs= 1871; DryHrs= 6165; PTOF= 532.20]
02527 *****
02528 Maximum average rainfall intensities over
02529 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02530 19.50 13.55 9.03 5.42 2.93 1.84 1.32 1.02 .70 mm/hr
02531 18.50 12.50 8.00 4.50 2.50 1.50 1.00 0.75 0.50 mm
02532 19.50 13.55 9.03 5.42 2.93 1.84 1.32 1.02 .70 mm/hr
02533 19.50 13.55 9.03 5.42 2.93 1.84 1.32 1.02 .70 mm/hr
02534 Number of rainfall events per following interval time
02535 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02536 1.32 1.04 .93 .71 .59 .43 .36 .31 .24
02537 Number of events with at least the following durations
02538 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02539 1.31 .72 .50 .19 .2 1 0 0 0
02540 # R0996.C0002-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02541 COMPUTE API
02542 [APIIn= 50.00; APIkdy= 9000; APIkdt= 9956]
02543 [APIave= 63.22; APIave= 19.39; APImin= .71]
02544 # Pre Development Condition - Using NASHHYD and CN
02545 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02546 # R0996.C0004-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02547 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.35 .154 1996.0731.1645 74.44 145 .000
02548 [CN= 65.0; N= 3.00; Tpe= 1.47]
02549 [IARCE= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02550 [InterEventTime= 12.00]
02551 # R0996.C0005-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02552 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.85 .157 1996.0731.1630 72.91 142 .000
02553 [CN= 61.0; N= 3.00; Tpe= 1.22]
02554 [IARCE= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02555 [InterEventTime= 12.00]
02556 # R0996.C0006-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02557 ADD HYD + 15.0 02:EstPrc 39.85 .157 1996.0731.1630 72.91 n/a .000
02558 [CN= 61.0; N= 3.00; Tpe= 1.22]
02559 [IARCE= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02560 [InterEventTime= 12.00]
02561 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02562 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02563 # R0996.C0007-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02564 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.85 .157 1996.0731.1630 72.91 142 .000
02565 [CN= 61.0; N= 3.00; Tpe= 1.47]
02566 [IARCE= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02567 [InterEventTime= 12.00]
02568 # R0996.C0008-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02569 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.85 .157 1996.0731.1615 172.97 n/a .000
02570 [CN= 61.0; N= 3.00; Tpe= 1.22]
02571 [IARCE= 6.00; SMIN= 9.24; SK= .025]
02572 [InterEventTime= 12.00]
02573 # R0996.C0009-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02574 ADD HYD + 15.0 02:EstPrc 39.85 .157 1996.0731.1615 172.97 n/a .000
02575 [CN= 61.0; N= 3.00; Tpe= 1.22]
02576 [IARCE= 6.00; SMIN= 9.24; SK= .025]
02577 [InterEventTime= 12.00]
02578 # STORMS
02579 *****
02580 ** END OF RUN : 96
02581 *****
02582 *****
02583 *****
02584 *****
02585 *****
02586 *****
02587 *****
02588 *****
02589 # R0996.C0001-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02590 # R0996.C0001-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02591 START
02592 [ITER= .00 hrs on 19970101]
02593 [METOUT= 2 (1=Imperial, 2=metric output)]
02594 [NFORM= 0]
02595 [NRUN= 097]
02596 *****
02597 # SWMHYD Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02598 *****
02599 # Project Name : [Cavain Strittville West properties]
02600 # Project Number : [2267]
02601 # Date : [2021/12/14]
02602 # Modeler : [JB]
02603 # Company : [J.F. Sabourin and Associates]
02604 # License # : [2549237]
02605 *****
02606 # Ottawa International Airport (1967 - 2003)
02607 # READ AES DATA
02608 [Filename = 610600.123
02609 [Start_date = 1999.0101; End_date = 1999.1231]
02610 [DTF = 60; min: Length= 6040; hrs: WetHrs= 1799; DryHrs= 7661; PTOF= 433.20]
02611 *****
02612 Maximum average rainfall intensities over
02613 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02614 12.50 7.40 5.67 4.43 2.91 1.88 1.12 .84 .63 mm/hr
02615 12.50 15.20 17.00 26.60 34.90 40.40 40.40 40.40 45.30 mm
02616 19.50 12.50 8.00 4.50 2.50 1.50 1.00 0.75 0.50 mm
02617 19.50 12.50 8.00 4.50 2.50 1.50 1.00 0.75 0.50 mm
02618 Number of rainfall events per following interval time
02619 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02620 1.13 .92 .89 .67 .65 .55 .48 .43 .39
02621 Number of events with at least the following durations
02622 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02623 1.12 .70 .46 .20 .4 0 0 0 0
02624 # R0999.C0003-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02625 COMPUTE API
02626 [APIIn= 50.00; APIkdy= 9000; APIkdt= 9956]
02627 [APIave= 63.22; APIave= 19.39; APImin= .71]
02628 # Pre Development Condition - Using NASHHYD and CN
02629 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02630 # R0999.C0004-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02631 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.35 .103 1997.0221.2145 51.06 118 .000
02632 [CN= 65.0; N= 3.00; Tpe= 1.47]
02633 [IARCE= 6.00; SMIN= 54.78; SMAX=365.23; SK= .025]
02634 [InterEventTime= 12.00]
02635 # R0999.C0005-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02636 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.85 .100 1997.0222.1145 49.93 115 .000
02637 [CN= 61.0; N= 3.00; Tpe= 1.22]
02638 [IARCE= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02639 [InterEventTime= 12.00]
02640 # R0999.C0006-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02641 ADD HYD + 15.0 02:EstPrc 39.35 .103 1997.0221.2145 51.06 n/a .000
02642 [CN= 61.0; N= 3.00; Tpe= 1.22]
02643 [IARCE= 6.00; SMIN= 64.50; SMAX=430.01; SK= .025]
02644 [InterEventTime= 12.00]
02645 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02646 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02647 # R0999.C0007-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02648 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.35 .103 1997.0221.2130 142.50 329 .000
02649 [CN= 61.0; N= 3.00; Tpe= 1.47]
02650 [IARCE= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02651 [InterEventTime= 12.00]
02652 # R0999.C0008-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02653 CONTINUOUS NASHHYD 15.0 01:EstPrc 39.85 .103 1997.0221.2115 142.50 329 .000
02654 [CN= 61.0; N= 3.00; Tpe= 1.22]
02655 [IARCE= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02656 [InterEventTime= 12.00]
02657 # R0999.C0009-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02658 ADD HYD + 15.0 02:EstPrc 39.85 .103 1997.0221.2115 142.50 n/a .000
02659 [CN= 61.0; N= 3.00; Tpe= 1.22]
02660 [IARCE= 6.00; SMIN= 1.39; SMAX= 9.24; SK= .025]
02661 [InterEventTime= 12.00]
02662 # STORMS
02663 *****
02664 ** END OF RUN : 97
02665 *****
02666 *****
02667 *****
02668 *****
02669 *****
02670 *****
02671 *****
02672 *****
02673 # R0999.C0001-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02674 # R0999.C0001-----Dtain-ID:INHYD-----AREAh-QFEARms-TpeakDate_hh:mm-----RvM-R.C-----DWfms
02675 START
02676 [ITER= .00 hrs on 19980101]
02677 [METOUT= 2 (1=Imperial, 2=metric output)]
02678 [NFORM= 0]
02679 [NRUN= 098]
02680 *****
02681 # SWMHYD Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
02682 *****
02683 # Project Name : [Cavain Strittville West properties]
02684 # Project Number : [2267]
02685 # Date : [2021/12/14]
02686 # Modeler : [JB]
02687 # Company : [J.F. Sabourin and Associates]
02688 # License # : [2549237]
02689 *****
02690 # Ottawa International Airport (1967 - 2003)
02691 # READ AES DATA
02692 [Filename = 610600.123
02693 [Start_date = 1998.0101; End_date = 1998.1231]
02694 [DTF = 60; min: Length= 5088; hrs: WetHrs= 2911; DryHrs= 4797; PTOF= 440.30]
02695 *****
02696 Maximum average rainfall intensities over
02697 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02698 15.80 8.90 7.60 4.00 2.54 1.82 1.27 .95 .76 mm/hr
02699 15.80 17.80 22.80 24.00 20.50 43.40 45.20 45.80 54.60 mm
02700 15.80 17.80 22.80 24.00 20.50 43.40 45.20 45.80 54.60 mm
02701 *****
02702 *****
02703 *****
02704 *****
02705 *****
02706 *****
02707 *****
02708 *****
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02899 *****
02900 *****

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02881 # Pre Development Condition - Using NASHHYD and CN
02882 #####
02883 R0100:C00004-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02884 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.35 .359 2000.0625_11:00 85.69 160 .000
02885 [Cm: 61.0; N: 3.00; Tpe: 1.47]
02886 [IAREC: 6.00; SMIN: 54.78; SMAx:365.23; EK: .025]
02887 InterEventTime: 12.00]
02888 R0100:C00005-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02889 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.85 .369 2000.0625_10:45 84.05 157 .000
02890 [Cm: 61.0; N: 3.00; Tpe: 1.22]
02891 [IAREC: 6.00; SMIN: 64.50; SMAx:430.01; EK: .025]
02892 InterEventTime: 12.00]
02893 R0100:C00006-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02894 ADD HYD 15.0 02:InfEstPre 39.85 .369 2000.0625_10:45 84.05 n/a .000
02895 + 15.0 02:InfEstPre 39.35 .359 2000.0625_11:00 85.69 n/a .000
02896 SIM: 15.0 01:InfPre 79.20 .728 2000.0625_10:45 84.87 n/a .000
02897 #####
02898 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
02899 #####
02900 R0100:C00007-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02901 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.35 .863 2000.0625_10:45 192.84 360 .000
02902 [Cm:100.0; N: 3.00; Tpe: 1.47]
02903 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
02904 InterEventTime: 12.00]
02905 R0100:C00008-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02906 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.85 .960 2000.0625_10:30 192.84 360 .000
02907 [Cm:100.0; N: 3.00; Tpe: 1.22]
02908 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
02909 InterEventTime: 12.00]
02910 R0100:C00009-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02911 ADD HYD 15.0 02:InfEstPre 39.85 .960 2000.0625_10:30 192.84 n/a .000
02912 + 15.0 02:InfEstPre 39.35 .863 2000.0625_10:45 192.84 n/a .000
02913 SIM: 15.0 01:InfPre 79.20 1.820 2000.0625_10:30 192.84 n/a .000
02914 #####
02915 # STORMS
02916 #####
02917 ** END OF RUN : 101
02918 #####
02919 #####
02920 #####
02921 #####
02922 #####
02923 #####
02924 #####
02925 RUN:COMMAND#
02926 R0102:C00001-----
02927 START
02928 [TZSR = .00 hrs on 20020101]
02929 [METOUT= 2 (1=imperial, 2=metric output)]
02930 [METORM= 0]
02931 [NRUN = 0102]
02932 #####
02933 # SWMHYD Ver:5.02/Jan 2001 <BETA / INPUT DATA FILE
02934 #####
02935 # Project Name : [Calvan Brittsville West properties]
02936 # Project Number: [2267]
02937 # Date : [2021/12/14]
02938 # Modeller : [JB]
02939 # Company : [J.F. Sabourin and Associates]
02940 # License # : [2549237]
02941 #####
02942 # Octava International Airport (1967 - 2003)
02943 R0102:C00002-----
02944 * READ AED DATA
02945 [Filename = 6106000.123]
02946 [Start_date= 2002.0101; End_date= 2003.1231]
02947 [DT= 60;min; Length= 5088;hrs; Metrics= 104; Dryhrs= 4784; PTO= 551.50]
02948 Maximum average rainfall intensities over
02949 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02950 45.00 26.75 18.40 9.48 4.74 2.48 2.08 1.56 1.04 mm/hr
02951 45.00 23.50 15.20 7.60 3.80 2.00 1.40 1.00 0.50 mm
02952 20020627 20020627 20020627 20020627 20020627 20020627 20020628 20020629 date
02953 Number of rainfall events per following interevent time
02954 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02955 100 83 78 56 47 41 36 34 25
02956 Number of events with at least the following durations
02957 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
02958 100 59 53 13 5 0 0 0 0
02959 #####
02960 R0103:C00003-----
02961 COMPUTE API
02962 [APIIn: 50.00; APIKey= 9000; APIKtr= .9956]
02963 [APIInsc: 114.06; APIave: 26.37; APIIn: 4.40]
02964 #####
02965 # Pre Development Condition - Using NASHHYD and CN
02966 #####
02967 R0102:C00004-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02968 CONTINUOUS NASHHYD 15.0 01:EastPre 39.35 .783 2002.0627_15:15 159.97 .290 .000
02969 [Cm: 61.0; N: 3.00; Tpe: 1.47]
02970 [IAREC: 6.00; SMIN: 54.78; SMAx:365.23; EK: .025]
02971 InterEventTime: 12.00]
02972 R0102:C00005-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02973 CONTINUOUS NASHHYD 15.0 01:WestPre 39.85 .844 2002.0627_15:00 158.24 .287 .000
02974 [Cm: 61.0; N: 3.00; Tpe: 1.22]
02975 [IAREC: 6.00; SMIN: 64.50; SMAx:430.01; EK: .025]
02976 InterEventTime: 12.00]
02977 R0102:C00006-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02978 ADD HYD 15.0 02:WestPre 39.85 .844 2002.0627_15:00 158.24 n/a .000
02979 + 15.0 02:EastPre 39.35 .783 2002.0627_15:15 159.97 n/a .000
02980 SIM: 15.0 01:Pre 79.20 1.625 2002.0627_15:00 159.10 n/a .000
02981 #####
02982 # Pre Development Condition and CN - No INFILTRATION
02983 #####
02984 R0102:C00007-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02985 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.35 1.819 2002.0627_15:00 273.41 486 .000
02986 [Cm:100.0; N: 3.00; Tpe: 1.47]
02987 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
02988 InterEventTime: 12.00]
02989 R0102:C00008-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02990 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.85 2.169 2002.0627_14:45 273.41 486 .000
02991 [Cm:100.0; N: 3.00; Tpe: 1.22]
02992 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
02993 InterEventTime: 12.00]
02994 R0102:C00009-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
02995 Add hyd 15.0 02:InfEstPre 39.35 1.819 2002.0627_15:00 273.41 n/a .000
02996 + 15.0 02:InfEstPre 39.85 1.819 2002.0627_15:00 273.41 n/a .000
02997 SIM: 15.0 01:InfPre 79.20 3.961 2002.0627_14:45 273.41 n/a .000
02998 #####
02999 # STORMS
03000 #####
03001 ** END OF RUN : 102
03002 #####
03003 #####
03004 #####
03005 #####
03006 #####
03007 #####
03008 #####
03009 RUN:COMMAND#
03010 R0103:C00001-----
03011 START
03012 [TZSR = .00 hrs on 20030101]
03013 [METOUT= 2 (1=imperial, 2=metric output)]
03014 [NRUN = 0]
03015 #####
03016 # SWMHYD Ver:5.02/Jan 2001 <BETA / INPUT DATA FILE
03017 #####
03018 # Project Name : [Calvan Brittsville West properties]
03019 # Project Number: [2267]
03020 # Date : [2021/12/14]
03021 # Modeller : [JB]
03022 # Company : [J.F. Sabourin and Associates]
03023 # License # : [2549237]
03024 #####
03025 # Octava International Airport (1967 - 2003)
03026 R0103:C00002-----
03027 * READ AED DATA
03028 [Filename = 6106000.123]
03029 [Start_date= 2003.0101; End_date= 2003.1231]
03030 [DT= 60;min; Length= 4440;hrs; Metrics= 406; Dryhrs= 4034; PTO= 554.60]
03031 Maximum average rainfall intensities over
03032 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
03033 15.10 10.00 7.13 4.28 3.18 1.86 1.23 .84 .81 mm/hr
03034 15.10 20.00 21.40 25.70 38.20 44.40 44.90 40.10 58.30 mm
03035 20030111 20030111 20030101 20030101 20030101 20030101 20030101 20030101 20030101 date
03036 Number of rainfall events per following interevent time
03037 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
03038 144 127 144 15 8 25 15
03039 Number of events with at least the following durations
03040 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
03041 144 80 43 13 5 1 0 0 0
03042 #####
03043 R0103:C00003-----
03044 COMPUTE API
03045 [APIIn: 50.00; APIKey= 9000; APIKtr= .9956]
03046 [APIInsc: 72.10; APIave: 28.44; APIIn: 4.70]
03047 #####
03048 # Pre Development Condition - Using NASHHYD and CN
03049 #####
03050 R0103:C00004-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03051 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.35 .235 2003.0711_18:00 111.68 .201 .000
03052 [Cm: 61.0; N: 3.00; Tpe: 1.47]
03053 [IAREC: 6.00; SMIN: 54.78; SMAx:365.23; EK: .025]
03054 InterEventTime: 12.00]
03055 R0103:C00005-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03056 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.85 .244 2003.0711_17:45 109.00 .197 .000
03057 [Cm: 61.0; N: 3.00; Tpe: 1.22]
03058 [IAREC: 6.00; SMIN: 64.50; SMAx:430.01; EK: .025]
03059 InterEventTime: 12.00]
03060 #####

```

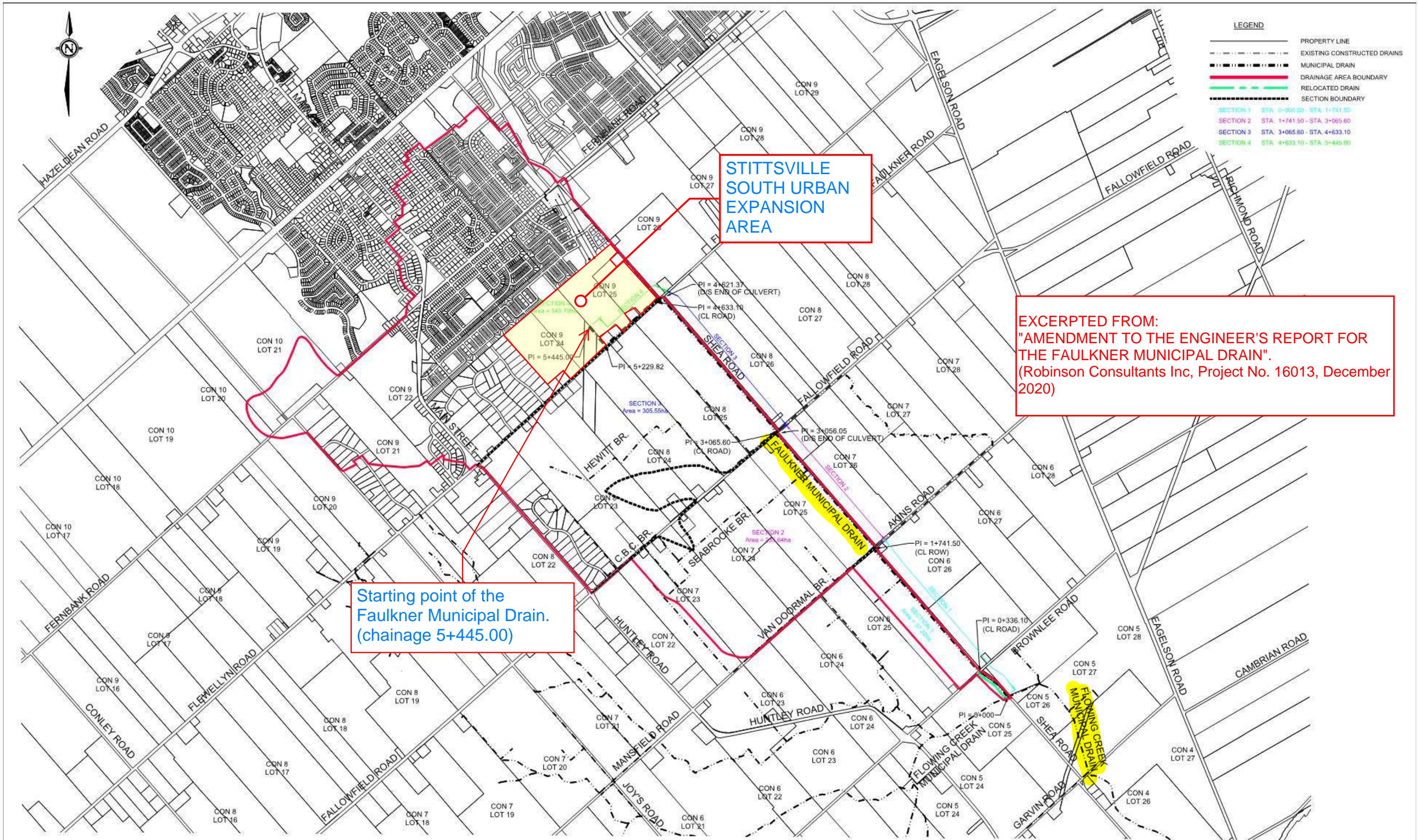
```

03061 R0103:C00006-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03062 ADD HYD 15.0 02:InfEstPre 39.85 .244 2003.0711_17:45 109.00 n/a .000
03063 + 15.0 02:EastPre 39.35 .235 2003.0711_18:00 111.68 n/a .000
03064 SIM: 15.0 01:Pre 79.20 79.200 2003.0711_17:45 110.33 n/a .000
03065 #####
03066 # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
03067 #####
03068 R0103:C00007-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03069 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.35 .778 2003.0711_17:45 231.08 417 .000
03070 [Cm:100.0; N: 3.00; Tpe: 1.47]
03071 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
03072 InterEventTime: 12.00]
03073 R0103:C00008-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03074 CONTINUOUS NASHHYD 15.0 01:InfEstPre 39.85 .903 2003.0711_17:30 231.08 417 .000
03075 [Cm:100.0; N: 3.00; Tpe: 1.22]
03076 [IAREC: 6.00; SMIN: 1.39; SMAx: 9.24; EK: .025]
03077 InterEventTime: 12.00]
03078 R0103:C00009-----Dtlm-ID:INHYD-----AREaha-QFEARcMs-TpeakDate_hh:mm-----RvMm-R.C-----DWFCms
03079 ADD HYD 15.0 02:InfEstPre 39.85 .903 2003.0711_17:30 231.08 n/a .000
03080 + 15.0 02:InfEstPre 39.35 .778 2003.0711_17:45 231.08 n/a .000
03081 SIM: 15.0 01:InfPre 79.20 1.663 2003.0711_17:45 231.08 n/a .000
03082 #####
03083 # STORMS
03084 #####
03085 #####
03086 FINISH
03087 #####
03088 #####
03089 WARNINGS / ERRORS / NOTES
03090 #####
03091 R0067:C00002 READ AED DATA
03092 *** WARNING: Requested start date is less than start date in file.
03093 *** WARNING: Missing rainfall increments were set to 0.
03094 *** WARNING: Missing rainfall increments were set to 0.
03095 *** WARNING: Missing rainfall increments were set to 0.
03096 *** WARNING: Requested start date is less than start date in file.
03097 *** WARNING: Missing rainfall increments were set to 0.
03098 *** WARNING: Missing rainfall increments were set to 0.
03099 *** WARNING: Missing rainfall increments were set to 0.
03100 *** WARNING: Missing rainfall increments were set to 0.
03101 *** WARNING: Missing rainfall increments were set to 0.
03102 *** WARNING: Missing rainfall increments were set to 0.
03103 *** WARNING: Missing rainfall increments were set to 0.
03104 *** WARNING: Requested start date is less than start date in file.
03105 *** WARNING: Missing rainfall increments were set to 0.
03106 *** WARNING: Missing rainfall increments were set to 0.
03107 *** WARNING: Missing rainfall increments were set to 0.
03108 *** WARNING: Missing rainfall increments were set to 0.
03109 *** WARNING: Missing rainfall increments were set to 0.
03110 *** WARNING: Missing rainfall increments were set to 0.
03111 *** WARNING: Missing rainfall increments were set to 0.
03112 *** WARNING: Requested start date is less than start date in file.
03113 *** WARNING: Missing rainfall increments were set to 0.
03114 *** WARNING: Missing rainfall increments were set to 0.
03115 *** WARNING: Requested start date is less than start date in file.
03116 *** WARNING: Missing rainfall increments were set to 0.
03117 *** WARNING: Missing rainfall increments were set to 0.
03118 *** WARNING: Missing rainfall increments were set to 0.
03119 *** WARNING: Requested start date is less than start date in file.
03120 *** WARNING: Missing rainfall increments were set to 0.
03121 *** WARNING: Missing rainfall increments were set to 0.
03122 *** WARNING: Missing rainfall increments were set to 0.
03123 *** WARNING: Missing rainfall increments were set to 0.
03124 *** WARNING: Missing rainfall increments were set to 0.
03125 *** WARNING: Missing rainfall increments were set to 0.
03126 *** WARNING: Requested start date is less than start date in file.
03127 *** WARNING: Missing rainfall increments were set to 0.
03128 *** WARNING: Missing rainfall increments were set to 0.
03129 *** WARNING: Requested start date is less than start date in file.
03130 *** WARNING: Missing rainfall increments were set to 0.
03131 *** WARNING: Requested start date is less than start date in file.
03132 *** WARNING: Missing rainfall increments were set to 0.
03133 *** WARNING: Requested start date is less than start date in file.
03134 *** WARNING: Missing rainfall increments were set to 0.
03135 *** WARNING: Requested start date is less than start date in file.
03136 *** WARNING: Missing rainfall increments were set to 0.
03137 *** WARNING: Requested start date is less than start date in file.
03138 *** WARNING: Specified end date is beyond the end date in file.
03139 *** WARNING: Missing rainfall increments were set to 0.
03140 Simulation ended on 2023-01-25 at 12:15:15]
03141 #####
03142 #####

```

Table B3 - Pre Development Water Budget

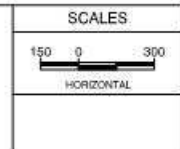
Year	Precipitation	Evaporation		Infiltration		Runoff	
	(mm)	(mm)	(%)	(mm)	(%)	(mm)	(%)
1967	386.9	213.7	55%	87.7	23%	85.5	22%
1968	592.8	356.7	60%	127.2	21%	108.9	18%
1969	569.8	359.1	63%	121.0	21%	89.7	16%
1970	558.9	358.6	64%	111.3	20%	89.0	16%
1971	522.1	361.4	69%	92.4	18%	68.3	13%
1972	784.3	441.3	56%	152.7	19%	190.3	24%
1973	744.9	435.8	59%	168.0	23%	141.1	19%
1974	386.2	281.2	73%	62.6	16%	42.4	11%
1975	535.5	338.7	63%	107.8	20%	89.0	17%
1976	492.4	348.6	71%	84.3	17%	59.5	12%
1977	677.6	430.6	64%	137.5	20%	109.5	16%
1978	638.8	408.5	64%	143.5	22%	86.9	14%
1979	866.5	466.3	54%	203.8	24%	196.4	23%
1980	622	411.9	66%	122.4	20%	87.8	14%
1981	936.4	536.6	57%	175.3	19%	224.5	24%
1982	596.1	407.7	68%	114.8	19%	73.6	12%
1983	587.3	412.8	70%	97.3	17%	77.3	13%
1984	459.4	280.8	61%	96.2	21%	82.4	18%
1985	559.9	341.9	61%	133.5	24%	84.5	15%
1986	849.4	500.1	59%	163.6	19%	185.8	22%
1987	639.9	441.6	69%	107.0	17%	91.3	14%
1988	643.2	418.5	65%	119.0	18%	105.8	16%
1989	522.5	356.7	68%	100.2	19%	65.6	13%
1990	727.8	462.7	64%	137.8	19%	127.3	17%
1991	555.8	398.5	72%	91.5	16%	65.8	12%
1992	730.2	456.3	62%	137.3	19%	136.5	19%
1993	721.1	486.1	67%	141.4	20%	93.6	13%
1994	527	322.4	61%	100.1	19%	104.5	20%
1995	321.6	162.9	51%	66.3	21%	92.4	29%
1996	512.2	339.2	66%	99.3	19%	73.7	14%
1997	433.2	290.7	67%	92.0	21%	50.5	12%
1998	440.3	297.8	68%	83.9	19%	58.6	13%
1999	424.4	273.4	64%	85.1	20%	65.9	16%
2000	535.9	343.1	64%	108.0	20%	84.9	16%
2002	551.5	278.1	50%	114.3	21%	159.1	29%
2003	554.6	323.5	58%	120.8	22%	110.3	20%
Average	589.1	370.7	63%	116.9	20%	101.6	17%
Min	321.6	162.9	50%	62.6	16%	42.4	11%
Max	936.4	536.6	73%	203.8	24%	224.5	29%



No.	DATE	REVISION	BY
1	15.04.20	ISSUED FOR MUNICIPAL REVIEW	AJR
2	27.05.20	ISSUED FOR AGENCY REVIEW	AJR
3	02.09.20	ISSUED FOR ECA APPLICATION	AJR
4	14.12.20	FINAL - ISSUED FOR DISTRIBUTION	AJR



Professional Engineers Ontario
 2012/14
 Licensed Engineering Technologist
 Name: L. Robinson
 Number: 100001000
 License: Professional 2012, 10-000010000
 Licensee's name and number shall be addressed under the Statute in force.



Robinson
 Consultants

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 Ottawa, Ontario K2V 1A8
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DESIGN	LF
CHECKED	AJR
DRAWN	JHB
CHECKED	LF
APPROVED	AJR

CITY OF OTTAWA
 FAULKNER
 MUNICIPAL DRAIN

MAINTENANCE SECTIONS and
 SECTION DRAINAGE AREAS

PROJECT No.	16013
CONTRACT No.	
DATED	DEC 2020
DWG. No.	FIG 6.1

The record of site-specific information associated with RVCA's regulatory approval process since 2006 was checked. It was found that no site-specific work affects the flood risk lines.

Drawings FL-1 and FL-2 in Appendix F depict the delineated floodplain and areas of shallow flooding.

7.2 Areas of Shallow Flooding

At a few places (Drawing FL-1), it is expected that flood water would go overbank from the channel and identifiable floodplain into the adjacent areas. However, there is considerable uncertainty as to how this would manifest itself and which area would actually be inundated by overbank water under the regulatory flood event. Many factors appear to be in the play: stream hydraulics, volume of flood water, volume of channel and floodplain storage, flood water escaping the stream as spills, amount of flood water available for spilling, local (micro) topography, shallow (sheet) flow with wet/dry fronts, rain water collected in depressions and puddles, soil and vegetation characteristics, etc. etc., and their interaction with each other. Needless to say, this is a very complex phenomenon and does not lend itself to engineering computation. While we can visualize the existence of areas of shallow flooding and can tentatively identify their probable extent, we cannot calculate with any degree of accuracy relevant parameters such as flood elevation, water depth or velocity. We tentatively call them areas of shallow flooding¹¹. Such areas of shallow flooding are prone to a lesser (lesser than identifiable floodplains) albeit unspecified degree of flood risk. Even though flood risk parameters cannot be estimated, the areas of shallow flooding are nonetheless hazardous to a certain extent. As such they fall –in our judgment – within the broad category of hazardous lands as defined

¹¹ Our concept of areas of shallow flooding is somewhat akin to that adopted by Credit Valley Conservation. A recent document (CVC 2010; Section 5.4.3) reads: “**Floodplain Spill Areas** – There are several areas within the CVC’s jurisdiction where floodplain spills occur. Spill areas are portions of the floodplain where hydraulic modeling and mapping of the riverine flood hazard indicates that flood waters are not physically contained within the valleyland and may or may not exit the watershed or subwatershed into surrounding lands. It is important to note that floodplain spill areas do not include the flood fringe, regardless of its characteristics such as flood flows and depths. Generally, the depth of flooding in spill areas cannot be readily determined as the flood depths that occur depend on a number of factors such as local and down-gradient topography, storage volume and the amount of spill flow that occurs. In addition, spills typically occur during higher flow rates of the storm event where the volume and depth of flood water is also dependent on the duration of the storm event.”

Drawing FL-1 Cross-sections and regulatory flood levels

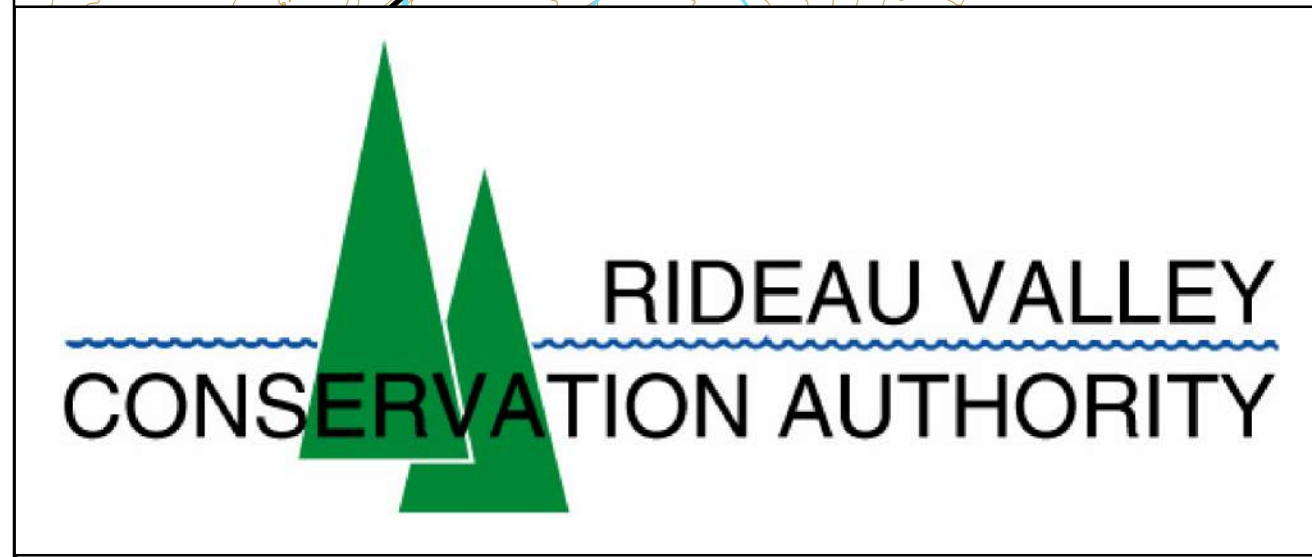
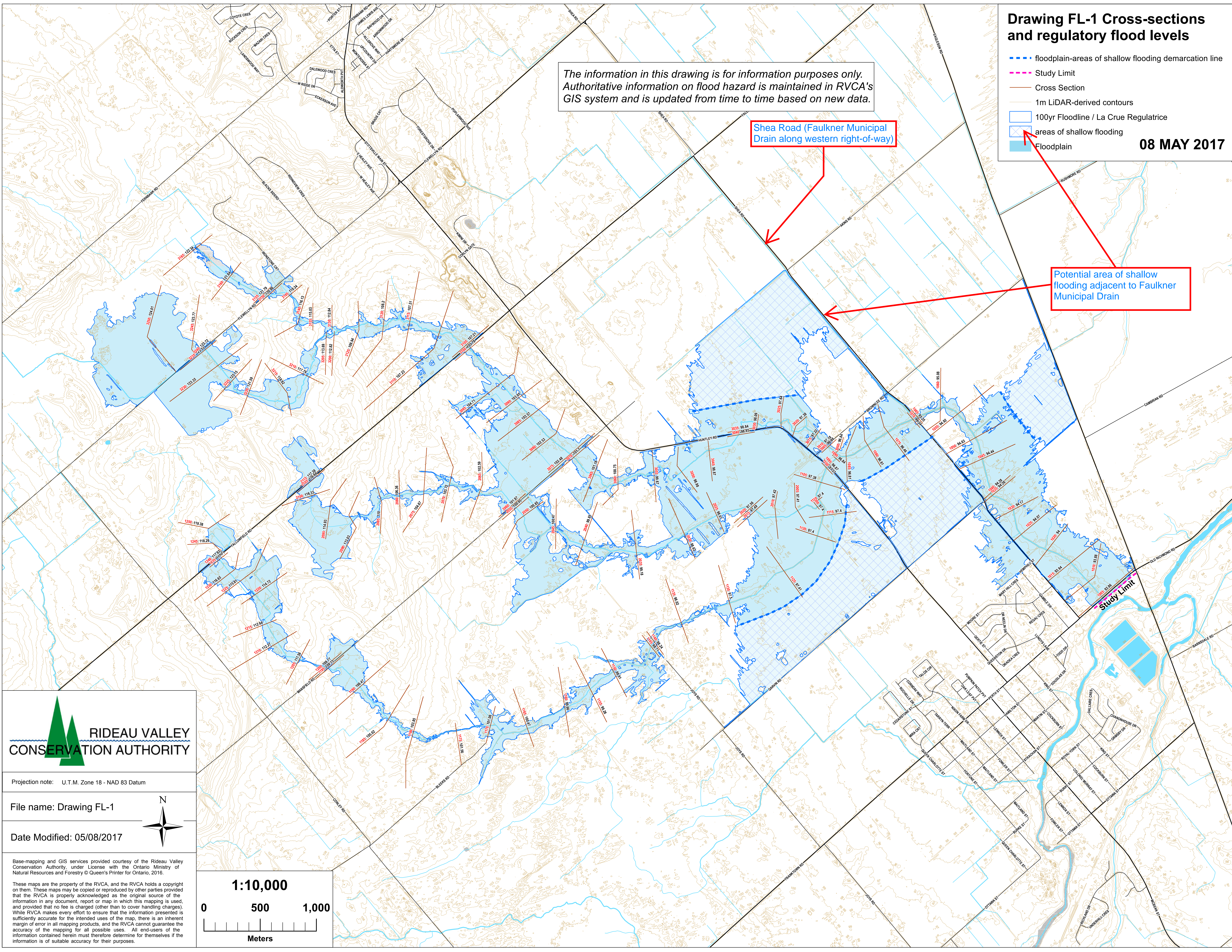
- - - floodplain-areas of shallow flooding demarcation line
- - - Study Limit
- Cross Section
- 1m LiDAR-derived contours
- 100yr Floodline / La Crue Regulatrice
- areas of shallow flooding
- Floodplain

08 MAY 2017

The information in this drawing is for information purposes only. Authoritative information on flood hazard is maintained in RVCA's GIS system and is updated from time to time based on new data.

Shea Road (Faulkner Municipal Drain along western right-of-way)

Potential area of shallow flooding adjacent to Faulkner Municipal Drain



Projection note: U.T.M. Zone 18 - NAD 83 Datum

File name: Drawing FL-1

Date Modified: 05/08/2017

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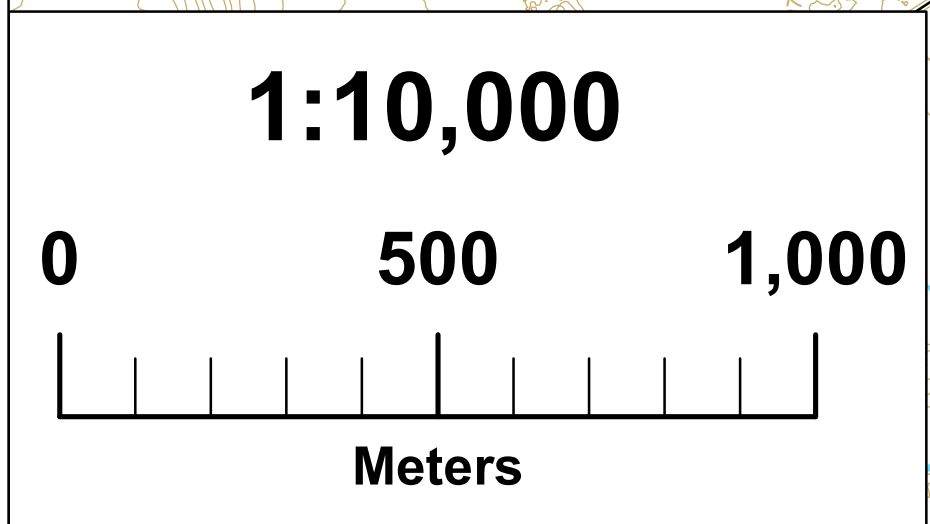
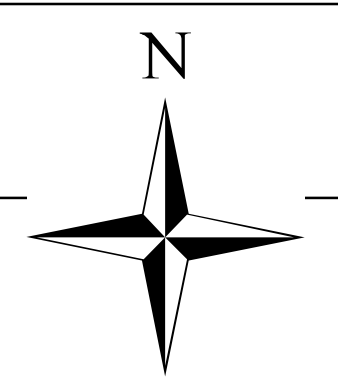


Table 1 - Groundwater Level Measurement Summary															
Well ID		BH1-21	BH2-21	BH3-21	BH22A-21	BH24-21	BH33-21	HA1-22	BH1-22	BH1A-22	BH2-22	BH3-22	BH3A-22	BH4-22	BH5-22
Ground Surface Elevation (m asl)		104.29	107.19	108.41	102.98	103.07	104.7	106.78	107.31	107.31	103.58	102.25	102.25	105.71	105.7
Groundwater (GW) Measurements															
11-Jan-22	GW Level (m bgs)	1.22	0.82	0.89	2.49	0.67	1.84	Wells Were Not Installed At This Time							
	GW Elevation (m asl)	103.07	106.37	107.52	100.49	102.40	102.86								
11-Oct-22	GW Level (m bgs)	1.12	1.16	0.90	2.61	0.60	2.12	0.31	1.33	1.44	1.52	0.84	0.81	3.62	1.62
	GW Elevation (m asl)	103.17	106.03	107.52	100.37	102.47	102.59	106.48	105.99	105.87	102.06	101.42	101.44	102.10	104.09
28-Oct-22	GW Level (m bgs)	1.01	0.95	0.92	N/A	0.46	1.98	0.28	1.35	1.43	1.52	0.61	0.40	3.65	1.64
	GW Elevation (m asl)	103.28	106.25	107.49	N/A	102.61	102.72	106.51	105.97	105.88	102.06	101.64	101.85	102.07	104.06

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Table 2 - Single Well Response Test Results Summary		
Test Hole ID	Screened Media	Hydraulic Conductivity (m/sec)*
HA1-22	Brown Silty Sand w/ Trace Gravel	1.51E-05
BH1-22	Bedrock	1.53E-05
BH1A-22	Brown Silty Sand/Glacial Till	Insufficient Water Volume to Conduct Test
BH2-22	Bedrock	8.99E-06
BH3-22	Bedrock	6.29E-05
BH3A-22	Brown Silty Sand/Glacial Till	4.48E-06
BH4-22	Bedrock	8.89E-07
BH5-22	Bedrock	1.52E-05
BH1-21	Bedrock	1.23E-04
BH2-21	Bedrock	3.99E-05
BH3-21	Bedrock	2.98E-06
BH22A-21	Bedrock	4.31E-07
BH24-21	Bedrock	6.19E-05
BH33-21	Bedrock	1.60E-04

*Average value from all tests conducted at each test location

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Table 3 - Overburden Field Saturated Hydraulic Conductivity Results and Estimated Infiltration Rates				
Test Completed Adjacent to Borehole ID	Infiltration Testing Elevation (m asl)	Material	K_{fs} (m/s)*	Unfactored Infiltration Rate (mm/hr)**
BH1-21	103.90	Brown Silty Sand	2.10E-06	56
	103.63	Brown Silty Sand	1.90E-06	56
BH2-21	106.95	Brown Silty Sand	6.40E-06	76
	106.65	Brown Silty Sand	5.30E-07	39
BH7-21	106.74	Brown Silty Sand	1.10E-06	47
	106.44	Brown Silty Sand	1.60E-06	52
BH11-21	104.68	Brown Silty Sand	2.70E-06	60
	104.38	Brown Silty Sand	1.60E-06	52
BH15-21	102.70	Brown Silty Sand to Sandy Silt	2.10E-07	31
	102.48	Brown Silty Sand to Sandy Silt	$\leq 8.1E-09$	≤ 13
BH17-21	106.74	Brown Silty Sand to Sandy Silt	5.90E-06	74
	106.44	Brown Silty Sand to Sandy Silt	4.10E-06	67
BH22-21	102.58	Brown Silty Sand	1.10E-06	47
	102.28	Brown Silty Sand	1.60E-06	52
BH23-21	102.33	Brown Silty Clay w/ Sand	5.30E-07	39
	101.70	Brown Silty Clay	$\leq 8.1E-09$	≤ 13
BH26-21	102.74	Brown Silty Clay w/ Sand	1.10E-07	26
	102.44	Brown Silty Clay w/ Sand	1.10E-07	26
BH29-21	101.87	Brown Silty Sand to Sandy Silt	5.30E-07	39
	101.57	Brown Silty Sand to Sandy Silt	2.70E-07	33
BH31-21	103.19	Brown Silty Sand to Sandy Silt	1.10E-06	47
	102.89	Brown Silty Sand to Sandy Silt	1.35E-07	27
BH37-21	103.21	Brown Silty Sand to Sandy Silt	5.30E-06	72
	102.91	Brown Silty Sand to Sandy Silt	5.90E-06	74

*Field hydraulic conductivity (K_{fs})

**The infiltration rates do not include a safety correction factor. Based on our testing results, a safety correction factor can range between 2.5 to ≥ 3.5 .

Table 4 - Horizontal Hydraulic Gradient Summary						
Well 'A'		Well 'B'		Distance (m)	Hydraulic Gradient (m/m)*	Date
Well ID	GW Elevation (m asl)	Well ID	GW Elevation (m asl)			
BH3-21	107.515	BH1-22	105.985	73	0.0208	October 11, 2022
BH3-21	107.515	BH1A-22	105.87	73	0.0224	October 11, 2022
BH3-21	107.515	BH5-22	104.085	131	0.0263	October 11, 2022
BH3-21	107.515	BH4-22	102.095	206	0.0263	October 11, 2022
BH1-22	105.985	BH2-21	106.03	197	-0.0002	October 11, 2022
BH1-22	105.985	BH1-21	103.17	442	0.0064	October 11, 2022
BH1-22	105.985	BH5-22	104.085	148	0.0128	October 11, 2022
BH1-22	105.985	BH2-22	102.06	447	0.0088	October 11, 2022
BH1A-22	105.87	BH2-21	106.03	197	-0.0008	October 11, 2022
BH1A-22	105.87	BH1-21	103.17	442	0.0061	October 11, 2022
BH1A-22	105.87	BH5-22	104.085	148	0.0120	October 11, 2022
BH1A-22	105.87	BH2-22	102.06	447	0.0085	October 11, 2022
BH1A-22	105.87	BH3A-22	101.44	708	0.0063	October 11, 2022
BH2-21	106.03	BH1-21	103.17	296	0.0097	October 11, 2022
BH2-21	106.03	BH2-22	102.06	358	0.0111	October 11, 2022
BH5-22	104.085	BH4-22	102.095	137	0.0145	October 11, 2022
BH5-22	104.085	BH2-22	102.06	330	0.0061	October 11, 2022
BH2-22	102.06	BH3-22	101.415	397	0.0016	October 11, 2022
BH2-22	102.06	BH3A-22	101.44	397	0.0016	October 11, 2022
BH33-21	102.585	BH3-22	101.415	485	0.0024	October 11, 2022
BH33-21	102.585	BH3A-22	101.44	485	0.0024	October 11, 2022
BH33-21	102.585	BH22A-21	100.37	549	0.0040	October 11, 2022
BH33-21	102.585	BH24-21	102.47	307	0.0004	October 11, 2022
BH3-22	101.415	BH22A-21	100.37	296	0.0035	October 11, 2022
BH3A-22	101.44	BH22A-21	100.37	296	0.0036	October 11, 2022
BH24-21	102.47	BH22A-21	100.37	524	0.0040	October 11, 2022
BH4-22	102.095	BH3-22	101.415	584	0.0012	October 11, 2022
BH4-22	102.095	BH3A-22	101.44	584	0.0011	October 11, 2022
BH4-22	102.095	BH33-21	102.585	404	-0.0012	October 11, 2022

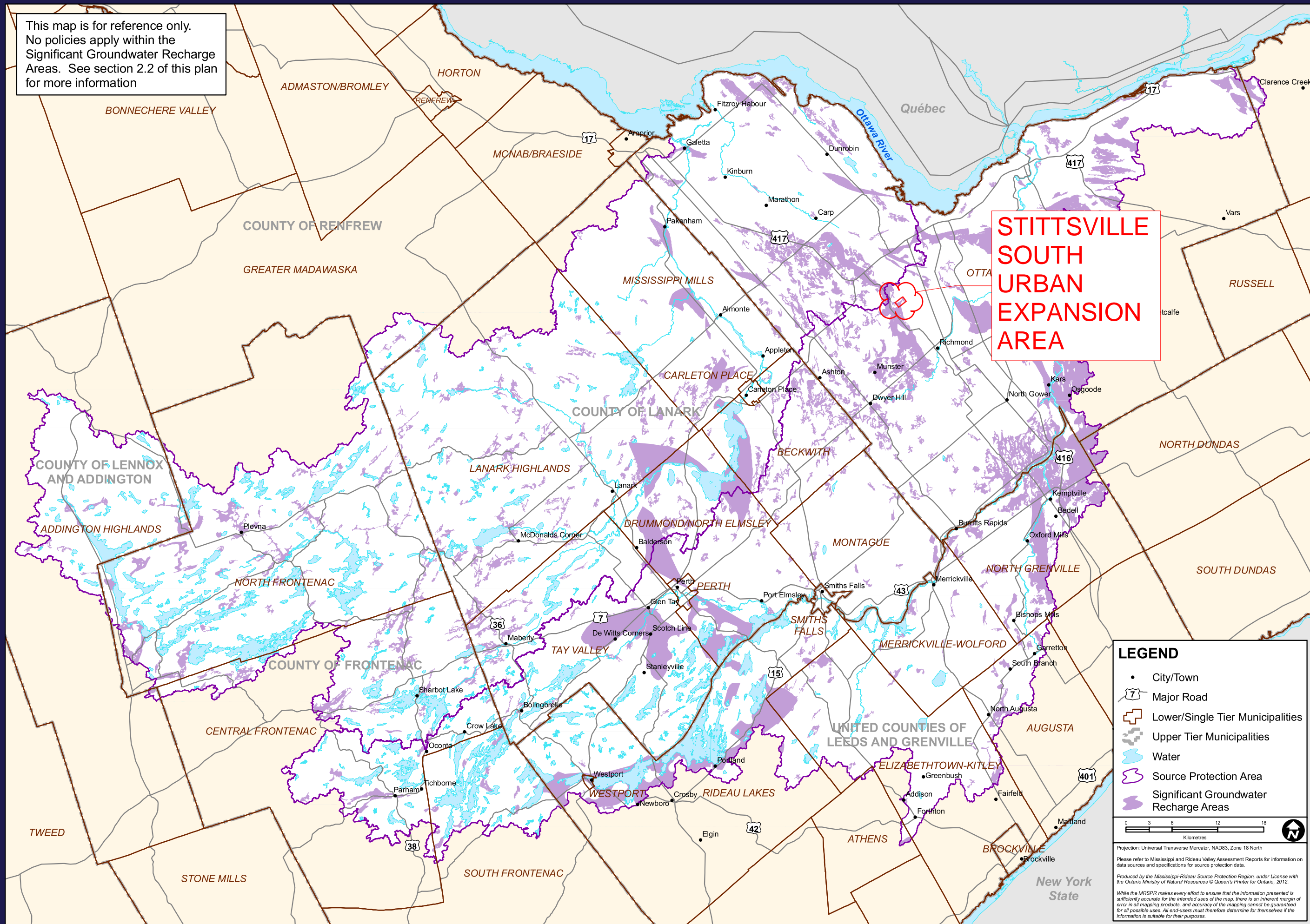
**Hydraulic Gradient = (GW Elevation Well 'A' - GW Elevation Well 'B') / Distance

Table 5 - Vertical Hydraulic Gradient Summary							
Well 'A'			Well 'B'			Hydraulic Gradient (m/m)*	Date
Well ID	GW Elevation (m asl)	Well Depth (m)	Well ID	GW Elevation (m asl)	Well Depth (m)		
BH1-22	105.985	98.29	BH1A-22	105.87	105.69	-0.0155	October 11, 2022
BH3-22	101.415	93.13	BH3A-22	101.44	99.1	0.0042	October 11, 2022
BH1-22	105.965	98.29	BH1A-22	105.88	105.69	-0.0115	October 28, 2022
BH3-22	101.64	93.13	BH3A-22	101.85	99.1	0.0352	October 28, 2022

*Hydraulic Gradient = (GW Elevation Well 'A' - GW Elevation Well 'B') / (Well Depth Well 'A' - Well Depth Well 'B')

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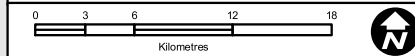
This map is for reference only. No policies apply within the Significant Groundwater Recharge Areas. See section 2.2 of this plan for more information



**STITTSVILLE
SOUTH
URBAN
EXPANSION
AREA**

LEGEND

- City/Town
- 7 Major Road
- Lower/Single Tier Municipalities
- Upper Tier Municipalities
- Water
- Source Protection Area
- Significant Groundwater Recharge Areas



Projection: Universal Transverse Mercator, NAD83, Zone 18 North
Please refer to Mississippi and Rideau Valley Assessment Reports for information on data sources and specifications for source protection data.

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APPENDIX E

TERMS OF REFERENCE



MEMORANDUM

DATE: June 9, 2022

TO: Christopher Rogers, P.Eng.

FROM: Kevin L. Murphy, P. Eng.

SUBJECT: Caivan – Stittsville Lands (5993, 6070 & 6115 Flewellyn Road)
New Urban Expansion Development – Terms of Reference (REVISED PER CITY
COMMENTS DATED MAY 6, 2022)

DSEL Job No. 21-1247

ATTACHMENTS:

Chris,

As per your request to Caivan at a pre-consultation meeting held (February 7/22), the following is a proposed summary of Terms of Reference (TOR) to document the servicing strategy approach for development of the above noted parcels of land located within Stittsville.

1.0 BACKGROUND

Fotenn Consultants Inc. (“Fotenn”) has previously circulated a January 27, 2022 outline for the development of Concept Plans and processes related to the above noted subject lands. Caivan Communities (“Caivan”) has ownership of land parcels that are currently located in the rural area and are designated to be brought within the urban boundary through the new Official Plan process.

1.1 Study Area & Objectives

The subject lands are bound by Flewellyn Road to south, Shea Road to the east, an existing urban subdivision development to the north (Stittsville South – Area 6) and an estate lot subdivision to the west. The area is also bisected by the Faulkner Municipal Drain and a Hydro One power transmission corridor.

The main objective of the servicing review is to develop an overall servicing strategy for the Stittsville Lands that will fulfill the requirements of municipal and provincial standards. The review will consider, evaluate and assess the servicing needs of the development area as it relates to geotechnical considerations, availability of service connections and stormwater management objectives. Several development alternatives for road network layouts, parks and unit mixes will be analyzed and assessed with respect to servicing strategies with a preferred overall servicing scheme identified.

The preferred internal servicing plan will be developed to meet regulatory requirements and will be free of conflicts between the various infrastructure components (water, wastewater, storm and stormwater infrastructure). The following sections present the anticipated scope of work to be completed:

- Task 1 (Agreement on Terms of Reference),
- Task 2 (Internal Concept Plan Review Process (Input Evaluation)),
- Task 3 (Functional Servicing Report and Master Infrastructure Review).

2.0 WORK PLAN

Task 1: Agreement on Terms of Reference

Preparation and finalizing of the TOR for the proposed servicing assessment approach guiding the Concept Plan development process. This draft TOR will be circulated to the City for review/comment on the proposed scope and will form the basis of the future servicability review.

Task 2: Internal Concept Plan Review Process (Input Evaluation)

From an Overall Servicing perspective, this task will include a thorough consolidation of the documents listed in Section 2.1, investigate and quantify residual capacities and servicing constraints while keeping in mind the environmental constraints identified as part of the Task 2 process. The scope of work to complete the Concept Plan Review Process will include the following components:

2.1 Review and Consolidate

As part of the Concept Plan Review Process, a review of background reports that are concerned with the study area will be completed. The review will, at minimum, include the following reports and guidelines being considered:

- City of Ottawa Sewer Design Guidelines (City of Ottawa, October 2012) & Technical Bulletins (ISDTB-2014-01, PIEDTB-2016-01, ISTB-2018-01, ISTB-2018-04 & ISTB-2019-02)
- City of Ottawa Water Distribution Guidelines (City of Ottawa, July 2010) & Technical Bulletins (ISD-2010-2, ISDTB-2014-2, ISTB-2018-02, & ISDTB-2021-03)
- Infrastructure Master Plan (City of Ottawa, 2013)
- Low Impact Development Technical Guidance Report, Implementation in Areas with Potential Hydrogeological Constraints (Dillon Consulting and Aquafor Beech, February 2021)
- Stormwater Planning and Design Manual (Ministry of the Environment, March 2003)
- Amendment to the Engineer's Report for the Faulkner Municipal Drain (Robinson, December 2020) & Addendum No. 1 (Robinson, March 2021)
- Stittsville South – Area 6, City of Ottawa, Master Servicing Report & Stormwater Management Design Plan (Novatech/DSEL, December 2013)
- Stittsville South Subdivision, City of Ottawa – Detailed Servicing & Stormwater Management Report (Novatech July 2016)
- Sanitary Pump Station Pre-Design Report, Stittsville South (Novatech, July 2015)
- Stittsville South Subdivision, City of Ottawa – Shea Road Sanitary Pump Station Design Brief (Novatech, May 2016)
- Design Brief, Davidson Lands – OPA 76 Area 6a, Phase 1 (5993 Flewellyn Road) (IBI Group, February 2018)
- Fernbank Community Design Plan, Master Servicing Study (Novatech, June 2009)

- Geotechnical Investigation, Proposed Residential Development 5993, 6070 & 6115 Flewellyn Road – Ottawa (Paterson (PG5570-2) January 2022)
- Low Impact Development Stormwater Management Guidance Manual (Ministry of the Environment, Conservation and Parks, Draft for Consultation – January 2022)

2.2 Hydrological Modelling

Based on a review of background reports and topographic information available, a hydrologic model will be developed to estimate peak flows and hydrographs under for the various outlets from the Study Area. The analysis will be conducted with SWMHYMO under the design storm types, return periods and hydrological parameters described in the Ottawa Sewer Design Guidelines. The analysis will consider the drainage features inventoried as part of the topographical survey (open ditch, culverts, etc.) as well as any drainage divides. Surface flows will be calculated based on the existing flow patterns for the various outlets; drainage ditches, culverts and storm sewers (if applicable).

2.3 Coordination and Liaise with Other Disciplines

The Concept Plan review process will discuss findings from other disciplines including geotechnical, hydrogeology, water budget, hydrology, ecology (aquatic resources and natural areas), etc., which will establish the existing environmental conditions. The coordination will ensure that the hydrologic analysis considers natural environmental inventories and constraints. Where required, drawings prepared will note existing conditions constraints and potential opportunities, which may impact storm and stormwater servicing or other municipal infrastructure.

2.3.1 Coordinate and Liaise with Geotechnical Engineer

In consultation with the geotechnical engineer, DSEL will:

- Review specific grade raise restrictions to better understand the potential grading constraints versus potential land use;
- Review the soil's characteristics to better understand whether they are conducive to infiltration measures;
- Review the soil's structural capabilities from a support/strength perspective;
- Review the areas of either recharge or discharge potential.

2.3.2 Coordinate with the Hydrogeologist

In consultation with the hydrogeologist, the existing conditions water budget analysis will be reviewed to identify the zones conducive to infiltration measures or other low impact development (LID) strategies. These measures could potentially be used to mitigate impacts on the water budget. As part of this task, LID strategies will be reviewed, at a conceptual level, to determine their viability and effectiveness in maintaining the existing conditions water budget and potential benefits to mitigating downstream impacts.

- Prepare a conceptual LID plan which illustrates the zones noted above and how the measures will be integrated into the overall plan(s);

2.3.3 Coordinate with Biologist

In consultation with the biologist, the environmental constraints will be further reviewed to better understand their sensitivity to various land uses and their proximity to Concept Plan elements. The objectives and targets from a storm discharge perspective will be based on the on-site environmental constraints as well as the limitations of the receiving watercourses.

2.3.4 Review Topographical Survey and Complete Inventory of Existing Infrastructure

Once all constraints have been compiled a further review of topographical surveys will be completed as well as the drainage patterns identified under current conditions. As part of this task, existing services and outlets will be inventoried for wastewater, water and stormwater. The assessment of residual capacities for existing services will also be reviewed. Any additional survey data will be obtained as required to supplement as-built information.

2.4 Evaluation and Assessment of Storm Design Criteria , Objectives and Pond Alternatives

Based on the findings of the natural resource inventories, storm criteria for both water quality and quantity will be established from a consensus with other disciplines and based on requirements prevalent in the Study Area. Once adopted by the consultant team, the storm criteria will be presented and confirmed by regulatory agencies. Review and comment on potential end-of-pipe solutions that would satisfy the storm criteria and the most suitable approach and siting (based on topography, soil type etc) for the Concept Plans. Based on the siting of the facilities, establish footprint of the end-of-pipe facilities in accordance with the guidance described in Section 4 of the MECP SWMPDM. This includes evaluation of potential capacity of the existing Area 6 SWM pond to optimize use of that infrastructure.

Pond sizing will be established conservatively and not be downsized based on the finding of LID options reviewed to establish water balance.

2.5 Coordination with Drainage Engineer for requirements relating to the Faulkner Municipal Drain.

Consultation will be undertaken with the City's Municipal Drain Group to assess any requirements under the Ontario Drainage Act for the Faulkner Municipal Drain in terms of drainage outlets and land use changes proposed. The consultation will serve as the basis for any amendments to the existing Engineer's Report that may be required.

2.6 Concept Plan Summary Discussions & Preferred Plan Selection

The preceding evaluations considered along with the Concept Plans reviewed will determine a preferred plan which will be brought forward for the more detailed review and assessment of servicing in Task 3.

Task 3: Functional Servicing Report and Master Infrastructure Review

After the completion of Task 2, the Consulting Team will have developed several Concept Plans based on the findings and any other discipline inputs compiled to date from the Team with a preferred option selected. This will include environmental, stormwater, geotechnical and transportation. For the preferred Concept Plan the municipal servicing constraints criteria (see Task 3.1) will be investigated for the preparation of the servicing analysis. Review will also include comment on suitable servicing routes via either servicing blocks and/or the establishment of right-of-way corridors that have appropriate cross-sections to accommodate the various elements of servicing infrastructure required.

3.1 Evaluation of Municipal Servicing Requirements for the Preferred Concept Plan

DSEL will evaluate infrastructure servicing alternatives for the Concept Plans prepared by considering each option and providing the Team with inputs using the general criteria outlined below in order to resolve the preferred Concept as described in the Fotenn outline memo previously circulated. The tasks envisioned to be included in a Site Servicing and Stormwater Management Report area as follows:

3.1.1 Grading

1. Develop a macro level Grading Plan for the Concept Plans based on the constraints identified by the geotechnical engineer. Grading will be developed in accordance with the criteria described in the Design Guidelines.

3.1.2 Identify and Assess Capacity of Existing Conveyance Systems

1. Based on topographical maps/surveys and servicing reports of existing developments adjacent to the limits of the Study Area, free flowing capacity of watercourses (i.e. Faulkner Drain), roadside ditches and water crossings (if any) will be reviewed.

3.1.3 Water Infrastructure

1. Confirm pressure objectives with the City along feeder mains under both domestic and fire flow conditions. Connections will be to the development areas to the north of the Study Area. Coordination with the Water Master Plan to be undertaken with City staff.
2. Calculate domestic demands (average, maximum day and peak hour) based on “system level parameters” (expectation being there will be in excess of 3,000 persons) under the build-out condition of the proposed land use for the selected Concept Plans. The preferred parameters will be provided by the City.
3. Calculate required design fire flow for concurrence by City staff.
4. Calculate theoretical domestic demands for potential phases of development based on a phasing strategy. Develop and populate a base water model for the preferred Concept Plan.
5. Acquire hydraulic boundary conditions at each of the connection points of the existing water distribution system. Proposed connection locations to be concurred with by City Staff.
6. Evaluate the performance of the distribution system against municipal requirements under domestic demand conditions for the Concept Plan. Assess and identify deficiencies and develop system upgrades, if required, to meet municipal requirements from both pressure and demand criteria.
7. Evaluate the performance of the proposed distribution system under a maximum day plus fire flow conditions for the Concept Plans supply characteristics of the pressure zone in accordance with Technical Bulletins.
8. Prepare a Water Servicing Plan for the preferred Concept Plan.

3.1.4 Wastewater Infrastructure

1. Based on the sanitary sewer outlets inventoried as part of Task 2, assess residual capacities. Coordination Wasterwater Master Plan to be undertaken with City Staff.
2. Develop peak wastewater flows based on the land use and population projections for the different land uses associated with the Concept Plans as per the Sewer Design Guidelines.
3. Prepare a Sanitary Drainage Area Plan and Design Sheets for the preferred Concept Plan.
4. Review trunk sanitary sewer routes, establish preliminary invert elevations based on topography and existing outlets. Prepare Sanitary Servicing Plan and assess impact of phasing on infrastructure. Identify servicing constraints, potential crossing conflicts and adjust, as required once the Storm Servicing Plan has been completed.
5. Assess residual capacities, beyond the Study Area population.
6. Review Shea Road Sanitary Pump Station for capacity and potential upgrades. Coordination with the Wastewater Master Plan Project Manager to be undertaken in order to assess conceptual pumping upgrates that will be required to accommodate the expansion area.:
 - a. Summarize the existing pump station parameters.
 - b. Review of potential component upgrades as well as overflow requirements.
 - c. Review electrical changes needed to accommodate higher HP pumps and high-level electrical overview.
 - d. Transient analysis review.

7. Summarize findings for Wastewater Component within reporting.

3.1.5 Storm Servicing and Stormwater Management

1. Based on the prior Task findings, confirm storm design criteria (quantity and quality) with the RVCA, MECP and the City and discuss potential impacts.
2. Review topographic survey and maps. Based on the storm sewer outlets inventoried as part of prior tasks, confirm outlet locations and inverts, and assess residual capacities and drainage patterns, etc.
3. Review existing conditions hydrological analysis to establish the baseline condition.
4. Finalize capacity assessment of existing surface outlets using desktop calculations.
5. Determine minor and major system drainage boundaries for the Concept Plans based on residual capacities of the existing outlets.
6. Carry out post-development Water Budget based on the Concept Plan. Identify and assess water budget deficits for the preferred Concept Plan.
7. In consultation with the hydrogeologist:
 - Investigate, at the conceptual level, the integration of low impact development (LID) strategies within the Study Area based on inputs from the hydrogeologist
 - evaluate potential infiltration measures, and
 - assess conceptually the performance of the LID strategies and infiltration measures with respect to the potential water budget deficits.
8. Based on the minor and major system boundaries, prepare post-development Storm Drainage Area Plan and Servicing Layout for the preferred Concept Plan. Identify servicing constraints, potential crossing conflicts and adjust, as required.
9. Coordinate with the City Drainage Group regarding the Faulkner Drain and any requirements under the Ontario Drainage Act.
10. Prepare Storm Sewer Design Sheets and Drainage Area Plans for the preferred Concept Plan with appropriate runoff coefficients, assessment of trunk storm sewer inverts etc as per Sewer Design Guidelines.
11. Review and finalize potential end-of-pipe solutions that would satisfy the storm criteria (water quality and quantity) and the most suitable approach and siting (based on topography, soil type etc) for the preferred Concept Plan. Based on the siting of the facilities, establish footprint of the end-of-pipe facilities in accordance with the guidance described in Section 4 of the MECP SWMPDM.
12. Carry out a hydraulic grade line (HGL) analysis of the proposed storm sewer system to evaluate the freeboard between the potential underside of footings and the 1:100 year storm. The analysis is to include the evaluation under the climate change event in accordance with the OSDG.
13. Assess impact of phasing on proposed storm infrastructure.
14. Summarize findings for Stormwater Management within the reporting

3.1.6 Water Budget

1. In consultation with the hydrogeological/geotechnical engineer, JFSA/DSEL will prepare a pre- and post-development water balance review (infiltration, runoff and evapotranspiration) for the site in accordance with the methodology summarized in Section 3.2 of the MECP's "Stormwater Management Planning & Design Manual, March 2003". This will include consideration of *Table 3.1 – Hydrologic*

Cycle Component Values and evaluation of 39 years of historical rainfall data from the Ottawa Airport via continuous hydrologic SWMHYMO model simulations. As per 4.7.1 (3.b) of the draft Official Plan.

2. Findings above will also be correlated to the mitigation of potential downstream impacts of the development.

3.1.7 Opinion of Probable Cost and Phasing

1. Coordination with other disciplines to finalize phasing for the Concept Plan in regard to servicing constraints.
2. Prepare an opinion of probable cost for municipal servicing for the preferred Concept Plan.

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